

Geotechnical Investigation & Slope Stability Assessment

Proposed Residential Subdivision – 229 Robson
Hursley Road, Torrington



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1. INTRODUCTION

This report presents the results of a geotechnical investigation carried out by Core Consultants Pty Ltd (Core) for a proposed residential subdivision of Lot 1 SP349893 at 229 Robson Hursley Road, Torrington.

The work was requested by Nexus Urban (Nexus) on behalf of Tunney Group in accordance with our proposal Q007129-001-L-Rev0 dated 17 April 2026.

Core have previously carried out a geotechnical investigation and slope stability assessment at the adjoining lot for a proposed industrial subdivision (Lot 34 SP312467 at 229 Robson Hursley Road, Torrington), refer report J002623-001-R-Rev1, dated November 2025.

This report includes a summary of site conditions and method of investigation along with comments and recommendations regarding earthworks, including site preparation, excavatability and batter slopes, dispersive soils, preliminary site classification to AS2870-2011, pavement design parameters and slope stability risk assessment.

2. PROPOSED DEVELOPMENT

Our understanding of the proposed development is based on plans provided by Nexus on 16 April 2026 (Job No.470, Drawing No.MP-01, CP-01, SK-01, SK-02 and SK05, Revision P).

The proposed residential subdivision will involve the development of one lot into 19 lots along with associated infrastructure including roads, retaining walls and services as shown in Image 1.

Based on available civil design information, bulk earthworks include cutting and filling of up to approximately 3 m (refer Image 2).

Plans provided indicate that batter slopes will be no steeper than 1V:4H; refer Image 3.

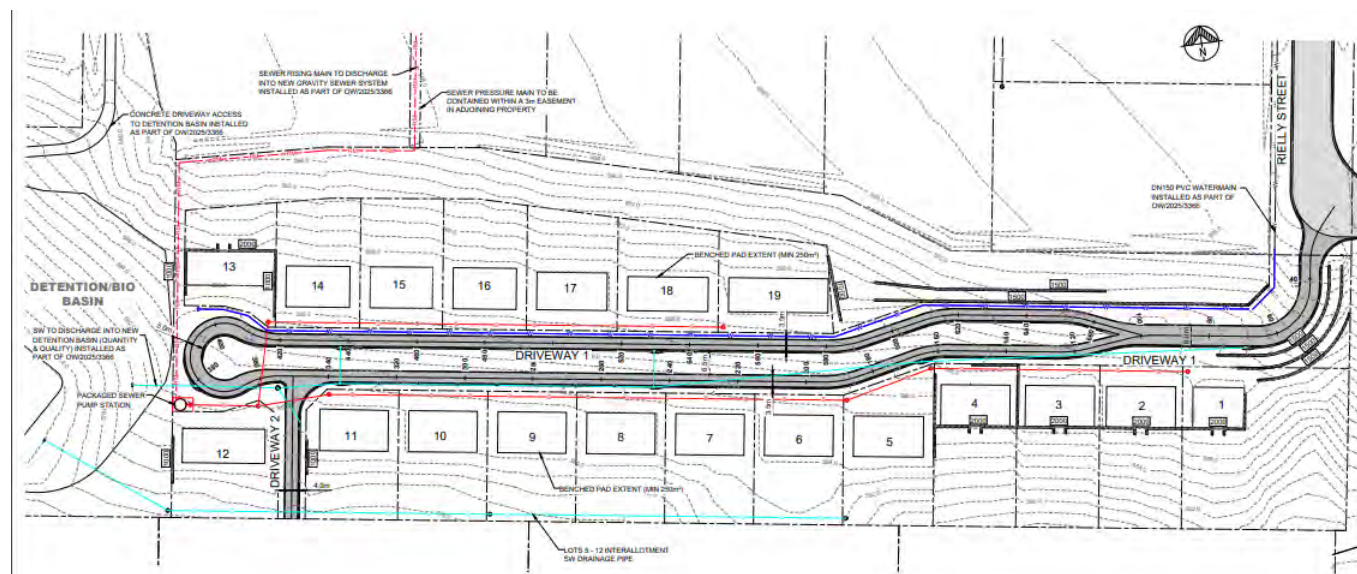


Image 1: Extract of Preliminary site plan (Job No.470, Drawing No.SK-01, Rev P)

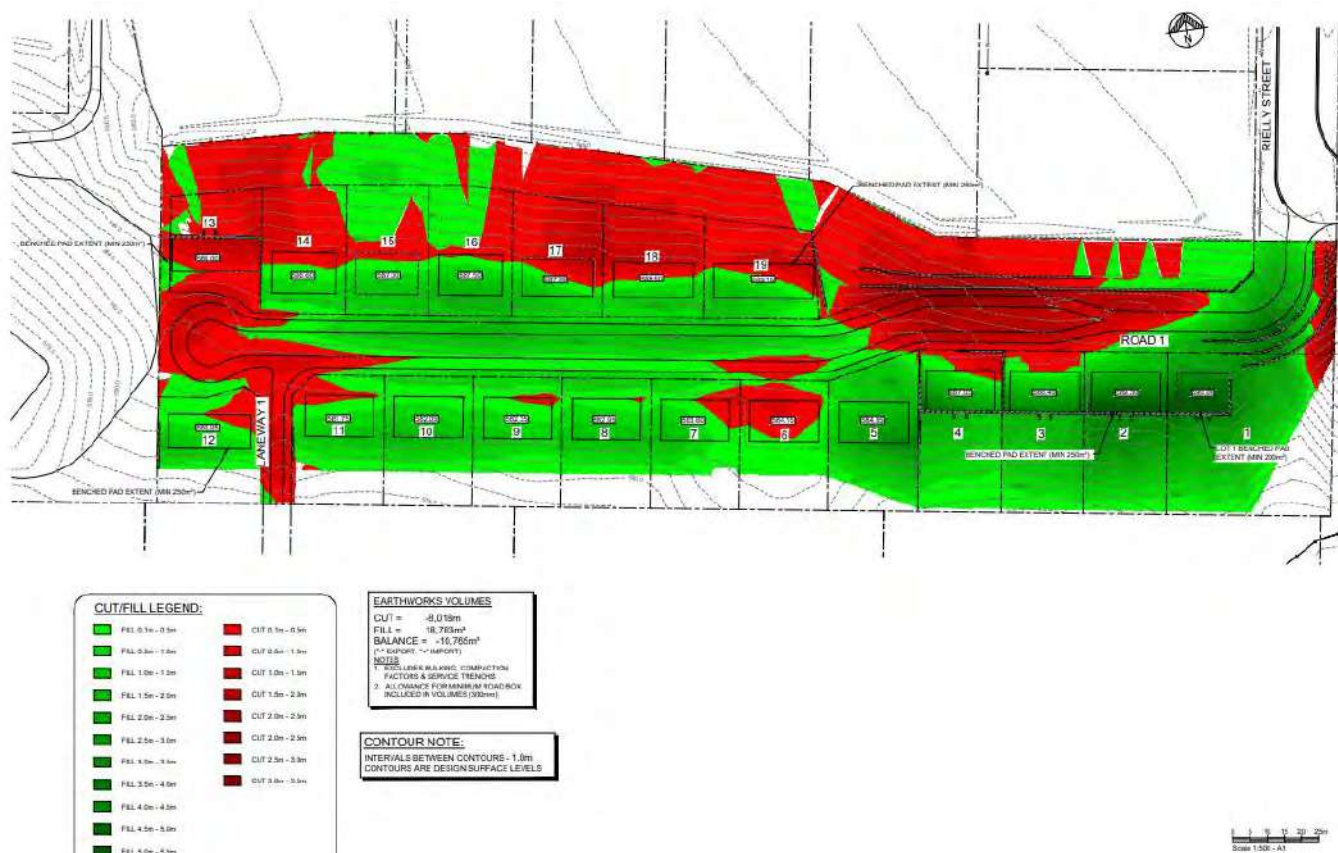


Image 2: Extract of Preliminary bulk earthworks plan (Job No.470, Drawing No.SK-02, Rev P)

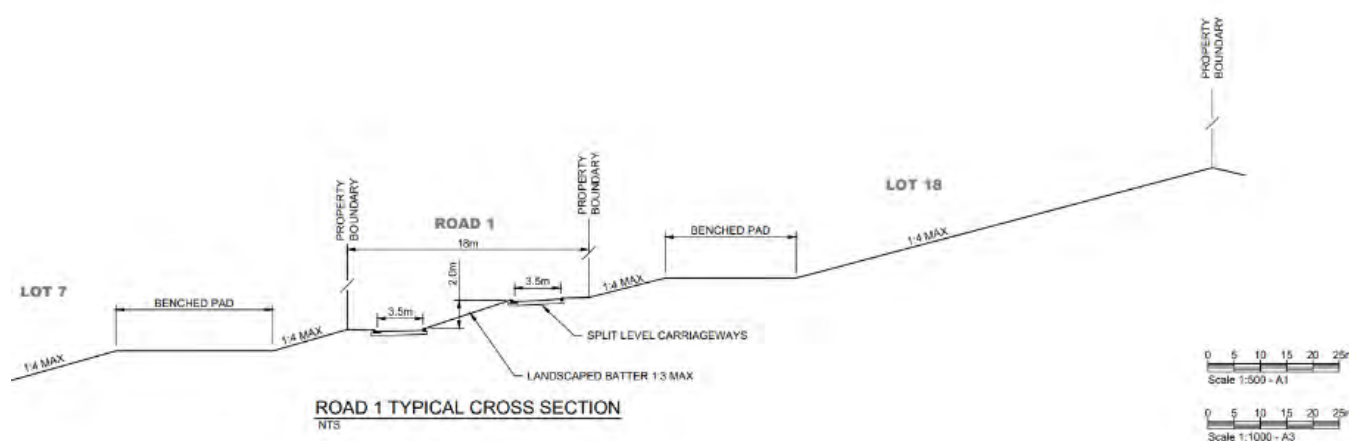


Image 3: Extract of Preliminary site plan (Job No.470, Drawing No.SK-01, Rev P)

3. SITE DESCRIPTION

The site is located at 229 Robson Hursley Road, Torrington between Rielly Street and Robson Hursley Road. It is bound by industrial subdivision (currently under construction in earthwork phase) to the north, vacant land to the east, rural-residential properties to the south and a drainage reserve to the west.

The site was accessed from Rielly Street and through the construction site to the north of the site. A track had been created for access down the slope and through the site.

At the time of the investigation on 14 May 2026, the site had been cleared of vegetation which had been stockpiled across the site. Ground coverage mostly comprised exposed soil with sections of grass and cobbles/boulders (refer Images 4 to 7).

The site predominately sloped down from the north to south with steeper slopes (up to 40°) along the northern boundary). Slopes tapered moving south across the site with slopes of approximately 10° recorded along the southern extents of the site.

General site conditions at the time of investigation on 14 May 2026 are shown in Images 4 to 7 below.



Image 4: General site conditions looking approximately southeast from the northwestern extents of the site.



Image 5: General site conditions looking southeast towards TP2



Image 6: General site conditions looking southeast towards TP5



Image 7: General site conditions looking south toward TP8

4. METHOD OF INVESTIGATION

4.1 Desktop Study

As part of our investigation, a desktop study was carried out comprising a review of previous projects in the area, published geological maps, the Toowoomba Region Council (TRC) maps and historical aerial imagery. This was followed by a site walkover and subsurface investigation.

4.2 Walkover and Subsurface Investigation

The field investigation including a walkover survey and subsurface investigation was carried out by a geotechnical engineer on 14 May 2026.

The walkover survey was conducted to assess the ground conditions and to make an appraisal of the general condition of the site. This included assessment of topography, drainage, vegetation cover, geology, erosion, and slope stability. Results of the walkover survey are discussed in Section 5.0.

The subsurface investigation comprised the excavation of eight test pits (designated TP1 to TP8).

Test pits were excavated with a 20-tonne excavator (organised by the client), fitted with a 1.2 m wide mud bucket and a 0.75 m wide toothed bucket to depths of up to 3 m below ground level (BGL). Dynamic Cone Penetrometer (DCP) tests were carried out adjacent to/within test pits to assess soil strength. Pocket penetrometer (PP) tests were carried out on cohesive materials.

Bulk disturbed and undisturbed U50 tube samples were collected from the test pits for subsequent laboratory testing.

The fieldwork was carried out by a geotechnical engineer from Core, who logged the subsurface conditions in accordance with Australian Standard AS1726-2017 *Geotechnical site investigations*.

Test locations were recorded in the field using a handheld GPS unit. Investigation locations are shown on the attached Figure 1 (Appendix A). Reports of Test Pits are attached in Appendix B, together with test pit photos and explanatory notes. Subsurface conditions are discussed in Section 5.2.

4.3 Geotechnical Laboratory Testing

Selected samples were forwarded for laboratory testing comprising:

- Atterberg limits and particle size distribution (percent fines)
- Emerson class number (ECN)
- Shrink swell index
- Single point-four day soaked California Bearing Ratio (CBR)

Results are discussed in Section 5.4 and attached in Appendix C.

5. RESULTS OF INVESTIGATION

5.1 Published Geological Information

Queensland geotechnical database (QGD) on-line mapping indicates the site is underlain by Eocene to Miocene age Main Range Volcanics consisting of “*olivine basalt*” (Tm).

The subsurface conditions encountered in the test pits generally confirm the published geology.

An extract from the geology mapping is provided in Image 8 below.



Image 8: Extract from Queensland Geological Database (QGD) with approximate site boundary.

5.2 Previous Projects

Core carried out a geotechnical investigation on the neighbouring site to the north in November 2024 (refer J002623-001-R-Rev1, dated November 2025). That investigation involved subsurface investigation and slope stability modelling.

5.3 Subsurface Conditions

The subsurface profile encountered in the test pits generally comprised topsoil over stiff to hard residual clays underlain by extremely weathered basalt which transitioned to moderately weathered, medium to very high strength basalt. The basalt generally became less fractured and stronger with depth. Groundwater seepage/inflow was not encountered during excavation in any of the test pits.

The subsurface conditions encountered in the test pits are summarised in Table 1 with further details provided on the Reports of Test Pits (Appendix B).

Table 1: Summary of subsurface conditions

Location ID	Material Depth Range (m)				
	Topsoil	Residual Soil	Extremely Weathered (XW) Basalt	Basalt	Groundwater
TP1	0 – 0.15	0.05 – 0.2	0.2 – 1	1 – 3*	NE
TP2	0 – 0.2	0.2 – 0.6	0.6 – 1.8	1.8 – 3*	
TP3	0 – 0.2	0.2 – 0.6	0.6 – 1.5	1.5 – 2.8^	
TP4	0 – 0.2	0.2 – 0.5	0.5 – 1	1 – 2.5^	
TP5	0 – 0.1	0.1 – 0.5	0.5 – 1.1	1.1 – 2.4^	
TP6	0 – 0.2	0.2 – 0.6	0.6 – 1.1	1.1 – 2.1^	
TP7	0 – 0.15	0.15 – 0.4	0.4 – 1.1	1.1 – 2.4^	
TP8	0 – 0.3	NE	0.3 – 0.7	0.7 – 1.8^	
Note: * Termination depth ^ Practical refusal with 22T excavator					

5.4 Geotechnical Laboratory Testing

Laboratory test results (Appendix C) generally confirm the field soil classifications, and have been considered in the recommendations provided in Section 7. Results are summarised in Table 2 below.

Table 2: Summary of geotechnical laboratory classification test results

Test ID	Sample Depth (m BGL)	Material Type (Soil Origin)	Atterberg Limits and Linear Shrinkage			% Fines	Weighted Plasticity Index	Soaked CBR ¹		Field Moisture Content (%)	Optimum Moisture Content (%)	Shrink Swell Index Iss (%)	Emerson Class Number (ECN)
			Liquid Limit (%)	Plasticity Index (%)	Linear Shrinkage (%)			CBR Value (%)	Swell (%)				
TP2	0.2-0.6	CLAY (RESIDUAL)	71	46	20	-	-	2	8	26.7	29	-	4*
TP4	0.2-0.5		80	56	22.5	94	5,544	-	-	-	-	-	4*
TP7	0.15-0.4	Silty CLAY (RESIDUAL)	69	48	17	-	-	2	8	21.6	24	4.2	4*
Notes: *Carbonate Present 1: CBR testing was undertaken on samples remoulded to 95% DDR (standard) at optimum moisture content													

5.5 Topography

The subject site is on the southern side of a hill with predominate topography sloping down from the north to the south. The northern boundary of the site runs along the top of a ridge line at an approximate elevation of 597 m AHD and slopes down to the south at between 20° and 40°. Slopes became less steep south across the site, where slopes were recorded between 8° and 10° and is approximately 578 m AHD in elevation (refer Image 9).

Slopes are steepest along the eastern portion of the northern boundary (around test pits TP2 and TP3), with slopes measured at 40° for the upper portion of the slope before gradually tapering off to 20° and 10° to the southern boundary. The western portion of the site slopes were measured generally around 20° along the northern boundary that tapered to 10° by the centre of the site.

A small ridge running north-south was located approximately in the centre of the site (test pit TP4). Slopes either side of the ridge ran southeast and southwest measured between 8° and 10°.

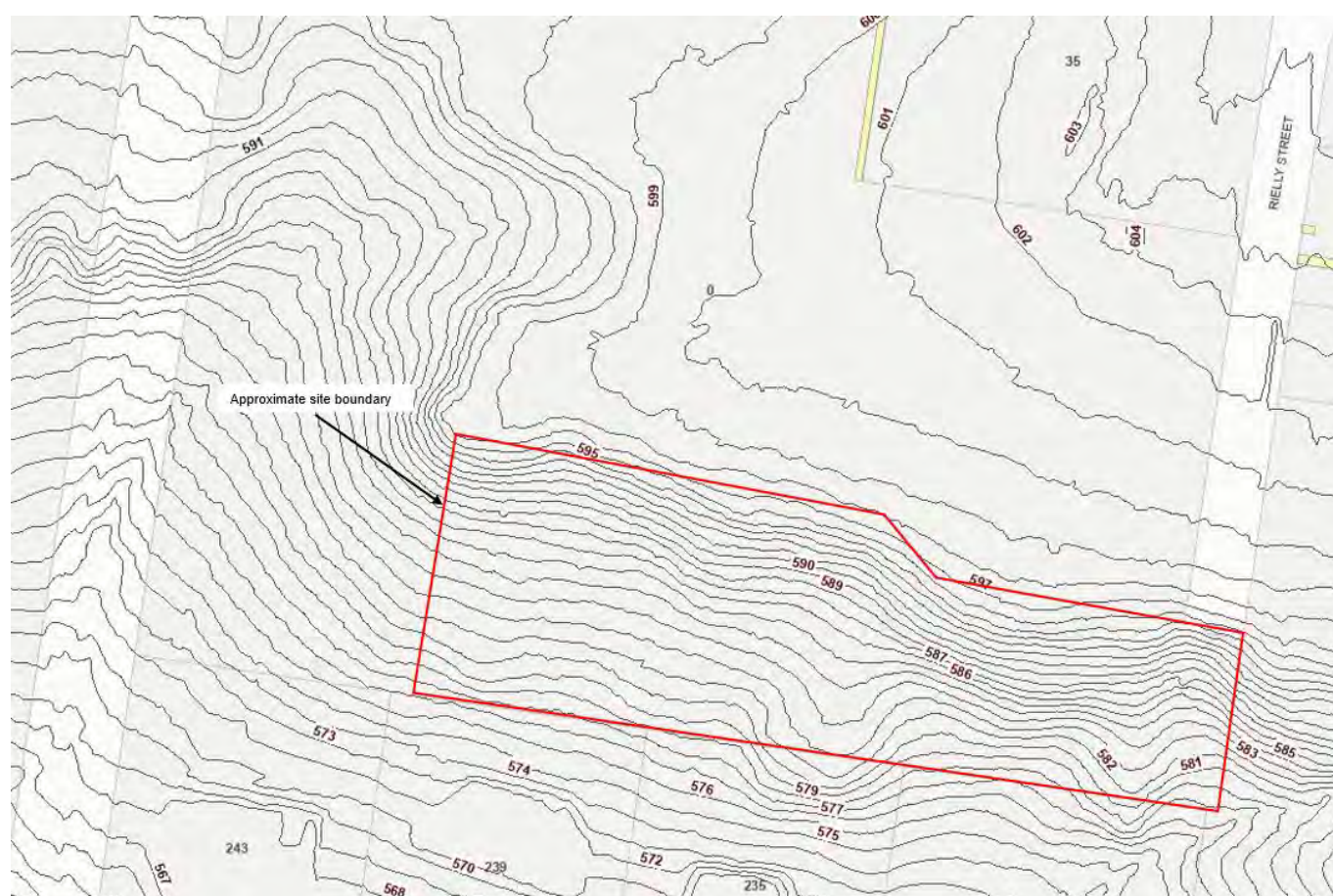


Image 9: Site contours (1 m intervals) from TRC online mapping.

5.6 Surface Drainage

Surface drainage appears to follow the site contours and for most of the site fall to the south to the neighbouring residential properties.

No scouring or natural waterways were observed at the time of investigation.

5.7 Vegetation

The site had been cleared of most vegetation including trees, bushes and grasses, leaving mainly exposed soils with some patches of dead grass.

5.7 Buildings, Other Structures, etc

No structures were located on the site.

6. SLOPE STABILITY RISK ASSESSMENT

A qualitative slope stability risk assessment for the proposed development has been carried out following the Australian Geomechanics Society (AGS) Guidelines for Landslide Risk Management (March 2007). The risk assessment relates to property and potential development, and is based on the proposed lot layout and earthworks provided to Core on 16 April 2026 (Job No.470, Drawing No.MP-01, CP-01, SK-01, SK-02 and SK05, Revision P).

Relative levels of risk and their implications are given in Table 3 below and the qualitative terminology (i.e. likelihood and consequence definitions) for use in assessing risk to property are attached in Appendix D.

Table 3: Stability Risk Levels

Risk Level		Example Implications ⁽¹⁾
VH	VERY HIGH RISK	Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of property.
H	HIGH RISK	Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.
M	MODERATE RISK	May be tolerated in certain circumstances (subject to regulators' approval) but requires investigation, planning and implementation of treatment options to reduce risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.
L	LOW RISK	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance required.
VL	VERY LOW RISK	Acceptable. Manage by normal slope maintenance procedures.

Note: (1) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide.

The risk assessment involves combining the elements of 'Likelihood' and 'Consequence' for the potential slope stability hazards identified across the proposed development area in a qualitative manner. Relative levels of risk and their implications are given in Table 4.

Table 4: Qualitative Risk Assessment for Property (Based on AGS 2007)

Hazard	Likelihood	Consequence	Assessed Risk	Comments
Shallow failure of soils above the footing zone	Unlikely	Medium	Low	The likelihood of shallow failure is assessed as <i>Unlikely</i> due to the soil strength, lack of groundwater and subject to implementation of the recommendations in this report. The potential consequence is considered <i>Medium</i> , for a resulting <i>Low</i> assessed risk.
Deep-seated failure through very stiff to hard residual soils; extremely weathered material or weathered rock	Rare	Major	Low	A likelihood of <i>Rare</i> is assessed for global instability due to the competent nature of the residual soils and rock, as well as lack of evidence of previous global instability. A consequence of <i>Major</i> has been assigned, with a resultant <i>Low</i> assessed risk.

Based on our slope stability assessment, it is our opinion that there is a **Low Risk** for the proposed development as described in the referenced design drawings, provided that the recommendations in this report are implemented. This level of risk is normally acceptable to regulators (i.e. Council).

7. GEOTECHNICAL COMMENTS & RECOMMENDATIONS

The potential impacts on slope stability of the proposed development have been considered, and the measures recommended below have been designed to mitigate those impacts. Building development should follow normal slope maintenance procedures and the document '*Some Guidelines for Hillside Construction*', which is attached in Appendix E.

7.1 Earthworks

Cut/fill earthworks of up to approximately 3 m (refer Image 2) are proposed for shaping of the site and creating level building pads. Localised excavations for building foundations and underground services may also be required.

Earthworks are also expected to include creating access tracks, stripping of topsoil and rootlets and removing of existing stockpiles.

Earthworks should be carried out in accordance with Australian Standard AS 3798-2007 *Guidelines on earthworks for commercial and residential developments*.

7.1.1 Site Preparation and Trafficability

At the time of the investigation the site was trafficable with a tracked excavator and 4WD vehicle. Experience from previous investigations on the neighbouring site has shown that wet conditions can cause boggy and slippery conditions across the site, especially where soils are exposed. It would be prudent to allow for the placement of a compacted granular fill layer particularly in access road and laydown areas. A geotextile separation layer between the natural ground and granular fill may also be required.

Prior to the placement of fill or construction at-grade, the following works are recommended:

- Strip the surface of vegetation, organic matter, organic rich soils (topsoils) and severely root affected soils (expected to range between about 0.1 m to 0.3 m BGL based on the results of the investigation). Localised deeper stripping/excavation may also be required where larger tree root systems need to be removed. Any organic rich topsoil must not be used as fill in proposed building and/or pavement areas, however it may be used for non-structural purposes (i.e. landscaping).
- 'Proof roll' the stripped surface by several passes of a minimum 10 tonne roller to identify loose or soft areas.

An experienced geotechnical engineer should carry out an inspection of the subgrade 'proof roll'. Loose/soft areas identified by proof rolling and/or inspection will require treatment by either of the following methods:

- Over-excavation and replacement with suitable fill compacted in layers; or
- Re-compaction of the loose or soft material provided that it does not contain substantial organic matter or deleterious substances. Moisture conditioning (e.g. drying) may be required, particularly if clayey subgrades have become exposed to water.

Additional measures to that would assist the earthworks phase could include:

- Benching of sloping surfaces. Also note that to control differential settlements across building pads to acceptable levels, additional benching may be needed to reduce the gradient of differential fill thicknesses.
- To reduce the risk of 'hard spots' below buildings and pavements, and to allow for future construction of shallow footings and underground services, the cut areas of the bulk excavation could be over excavated by at least 0.6 m below platform level (and ripped and compacted below that) and replaced with controlled fill.

It is recommended to undertake earthworks outside of the peak rainfall season, or allow for wet weather delays and other risk management measures associated with wet conditions.

7.1.2 Excavations

Excavations into the residual soils and extremely weathered rock should be achievable using conventional medium to large earthmoving equipment (e.g. 20 – 35 tonne excavators and D6 – D8 dozers). Alternatively, larger machinery could be used to improve production/efficiency rates, and in confined excavations such as trenching; a ripping tyne or hydraulic rock breakers may be required.

Larger earthmoving equipment with rock ripper or rock breaking attachments may be required for excavations into the medium strength (or stronger) rock (encountered as shallow as 0.7 m depth) or where rock mass is less fractured. Ripping direction should also be oriented to take best advantage of the fracturing orientation. Slow excavation productivity should be anticipated in the basalt.

7.1.3 Filling

The proposed bulk earthworks plan proposes filling of up to approximately 3 m.

It is recommended that filling is carried out with Level 1 supervision and testing in accordance with Australian Standard AS 3798-2007. Where filling is proposed, the following measures should be adopted:

- Imported fill should be compacted at moisture contents within the range of +2 to -2% of optimum moisture content for Standard Compaction.
- Confirmatory compaction testing must be carried out at regular intervals. Guidance on testing frequency is provided in AS 3798-2007 Table 8.1.
- Fill should have a maximum particle size of 100 mm.
- Fill should be compacted in layers, typically about 250 mm thick (loose thickness). However, layer thickness will be dependent upon the type/size of compaction plant, use of vibration, material type and condition.
- Fill should be compacted by repeated rolling to achieve a dry density ratio (DDR) of at least 95% of the Maximum Standard Dry Density for cohesive soils, or 70% Dry Density Index for cohesionless soils. Higher grades of compaction will be required in commercial development areas and for pavement sub-bases and base courses. A maximum DDR of 100% is to be adopted to reduce swell potential of the reactive clays.
- Imported fill should be cohesive in nature and have a plasticity index of < 15%, a maximum particle size of 100 mm, minimum fines content of 15%, and a soaked CBR of > 10%.
- Where fill is to be placed on sloping ground, the subgrade must be benched to assist with fill placement and compaction. Also note that to control differential settlements across building pads to acceptable levels, additional benching may be needed to reduce the gradient of differential fill thicknesses
- In accordance with good construction practice, fill embankments should be 'over built' then trimmed back to the well compacted material.

In poor subgrade areas, the use of geotextiles and/or rock/gravel blankets may be required.

Settlement of well compacted fill should be in the order of 0.5 % to 1% of fill thickness.

7.1.4 Material Re-use as Fill

The residual soils and weathered rock won from excavations on site, may be re-usable as general or structural fill - provided they are free from deleterious material, oversize particles (>100 mm size), and organic matter.

The following issues should be considered with regard to re-use of the site materials for structural fill:

- Topsoil and material containing significant organics must only be used for landscaping purposes.
- The residual soils contain a high proportion of silt/clay, and can be of high plasticity or reactive to changes in moisture variation. It is anticipated that difficulties with handling, placement, and compaction of these materials will be likely if it becomes too moist. It is recommended that this material be moisture conditioned to +1 to -2% of optimum moisture content (OMC) for compaction. Because of the high plasticity and low permeability, extended curing times may be required to achieve full moisture conditioning. Blending with the underlying, relatively less reactive, extremely weathered basalt, should produce a more workable material and decrease the overall reactivity to moisture variation.
- The residual clay materials are of high plasticity and may exhibit high shrink-swell movement, particularly if re-used as fill. The use of high plasticity residual clay soils should be avoided directly under proposed buildings and roads or other movement sensitive structures, and within upper fill layers. Alternatively, the natural highly reactive soils should be removed and replaced with inert imported fill or the spoil derived from the weathered basalt. It is important that the fill contains some fines (i.e., between approximately 15% and 30%) to prevent / reduce moisture ingress to the existing natural profile below.
- Rock excavation spoil may contain 'oversized' particles, comprising high strength (or stronger) basalt that might not break down easily during handling or compaction. Such oversize particles will need to be crushed or removed from excavation spoil prior to re-using as structural fill. This may be costly and time consuming.

7.1.5 Batter Slopes

For short-term construction periods, excavated un-surcharged faces less than 3 m high could be battered at 1V:1H in the residual very stiff (or stronger) clayey soils and weathered rock. Steeper batters could be formed temporarily

(nominally less than 1 week) under favourable weather conditions, subject to assessment of batter stability by a geotechnical engineer during earthworks.

Shoring will be required to enable personnel entry into near vertical sided trenches deeper than 1 m. Flatter batters may be required if significant groundwater seepage is encountered or if exposed faces are not protected from erosion.

Permanent batters should not be formed steeper than 1V:2H in the natural stiff (or stiffer) clays, extremely weathered rock, and controlled 'Level 1' fill (placed in accordance with the recommendations in Section 7.1.3). Permanent batters in medium or high strength rock may be constructed at 1V:1H (or possibly steeper), subject to inspection by a geotechnical engineer during excavation.

Further positive support by engineer-designed retaining structures may also be required where space restrictions prevent trimming of batters to safe slopes and where slopes are surcharged or near movement sensitive structures.

Permanent batters that are not retained should be protected from erosion (e.g. revegetation or stone pitching) as soon as possible after construction. Cut-off drainage at the crest of cut and fill embankments to intercept any surface water flow and direct it away from the slope and ensure there is drainage away from the toe of cuts and embankments.

Notwithstanding the above, routine maintenance of the batters may be required, particularly after heavy rainfall events.

The above recommendations do not supersede any existing safety regulations or legislation applicable to excavations (e.g. limits on personnel entry into trenches).

7.1.6 Erosion Protection

Any excavations or disturbance of materials should be carefully monitored and managed during the construction phase to minimise potential for scour/erosion and sediment run off. Laboratory test results for Emerson Class Number (ECN) i.e. soil dispersion indicate that the residuals soils tested are non-dispersive and have medium potential for erosion (ECN of 4).

Guidelines for the classification of dispersive soils are given in Ipswich Soil Management Guidelines (ISMG) and Brisbane City Council Notes and has been summarised in Table 5 below.

Table 5: Emerson Class Number Interpretation

Emerson Class Number	Erosion Potential
1 or 2	Very high potential for erosion
3	High potential for erosion
4	Medium potential for erosion
5	
6	
7 or 8	Low potential for erosion by dispersion process

Testing indicates that excavations will likely encounter non-dispersive soils, however the remoulding and breaking down of soils bonds can result in dispersive behaviour and as such all excavated materials should be treated as dispersive. Remoulding of the soil during filling and compaction operations (i.e., at moisture contents near optimum) does not increase the potential for dispersive behaviour. However, further breakdown of the soil may occur by water under turbulence or concentrated rapid flow. Under these circumstances, the 'non-dispersive' soils class of soil may be dispersive.

The following protection measures could be implemented to reduce the risk of soil erosion:

- Maintain existing topsoil and grass where stripping is not required (i.e., where no development is proposed).
- Stage clearing of existing vegetation and stripping of topsoil to limit periods of exposed soils.
- Exposed soil (i.e. on batters or drains etc.) should be protected from erosion by appropriate measures such as topsoil at least 150 mm deep, re-vegetation and hydro-seeding, protective matting, or hard facings as soon as practically possible following excavations.
- During construction if covering exposed soils is not practical, compact and 'seal' off exposed soils using several passes of a smooth drum roller when rainfall is forecast.

- Temporary sediment control should be adopted during construction where surface water will flow offsite e.g. along the southern boundary of the site.
- Concentrating surface runoff should be avoided unless adequate erosion protection and flow dissipation structures are constructed. Captured runoff is to be dissipated and spread over as wide an area as possible.
- Capping upper part of stormwater trenches with non-cracking/non-reactive and non-dispersive soils.
- Mixing of gypsum into soils (subject to close control of pH levels when in close proximity to waterways and other sensitive environments).
- Use of subsoil drains in major stormwater trench backfill to collect and carry seepage in a controlled manner into stormwater collection systems.
- Locate stockpiled material on level areas with appropriate bunds (compacted soils, silt fences, hay bales etc.) to limit runoff.

7.1.7 Retaining Walls

The design of flexible retaining walls (e.g. fully cantilevered) may be undertaken using a triangular pressure distribution and the earth pressure coefficients given below in Table 6.

Flexible walls are those which are free to rotate or tilt (i.e. cantilevered walls or single anchored or propped walls) and should be designed using an 'active' earth pressure coefficient (K_a). Where the walls are rigid and cannot rotate or tilt, then an 'at-rest' earth pressure coefficient (K_0) should be used.

Passive pressure should be ignored where there is potential for in-ground services trenches (or similar) in front of the wall.

Table 6: Lateral earth pressure coefficients

Retained Material	Bulk Density (kN/m³)	Friction Angle , Ø' (degrees)	Effective Cohesion c' (kPa)	Undrained Cohesion cu (kPa)	Lateral Earth Pressure Coefficients						Young's Modulus (MPa)	Poissons Ratio
					Ka				Kp	Ko		
					Angle of Retained Slope							
					0	10	20	30				
Fill (future)	*	*	*	*	*	*	*	*	*	*	*	*
Stiff clay	18	25	5	50	0.41	0.42	0.51	-	2.46	0.58	8	0.3-0.4
Very stiff clay	19	29	6	100	0.36	0.37	0.43	-	2.77	0.53	15	
Hard clay	20	30	8	200	0.31	0.32	0.35	0.5	3.25	0.47	30	0.2-0.3
Basalt low strength (or stronger)	21	40	-	-	0.22	0.22	0.24	0.27	4.6	0.36	75	
Notes: *Depends on fill material type and level of compaction												
Ka - active; Ko - at rest; Kp – passive												
For yielding walls active state develops when: Deflection > 0.001H to 0.004H (granular soil), or deflection > 0.01H to 0.04H (cohesive soil)												
Active earth pressure coefficients are calculated based on Coulomb earth pressure theory; with the assumption that there is zero friction between the wall and the backfill material.												

An allowance of 10 kPa (minimum) should also be made for lateral stress induced by compaction plant operating behind the walls. The effects of surcharge should be included by multiplying the vertical pressure developed by the surcharge by the appropriate lateral earth pressure coefficient from Table 6. Allowance should be made for sloping backfill if applicable.

Drainage material behind the wall should be installed for the full height of the wall, for a width of at least 0.3 m. The material must be free draining and granular and have a perforated or slotted drainage pipe at the heel of the wall to rapidly remove the water into the stormwater system. Alternatively, the wall will need to be designed for full hydrostatic pressure.

Footings for retaining walls should be founded in accordance with the recommendations in Section 7.2. The proposed founding materials should be assessed by a suitably qualified and experienced geotechnical engineer to confirm the required allowable bearing pressure has been achieved.

An assessment of global stability of proposed retaining walls should be carried out during the design process.

7.2 Foundations

7.2.1 Preliminary Site Classification

A site classification should be undertaken for each lot on completion of earthworks, in accordance with Australian Standard AS 2870 *Residential slabs and footings* which provides an indication of the likely magnitude of reactive (shrink and swell) movements (y_s) associated with normal seasonal soil moisture variations.

Shrink-Swell Index values have been estimated based on laboratory test results along with previous experience in the area with these materials. The adopted I_{ss} values are 5.2 % and 2.5 % for the residual soils and extremely weathered basalt, respectively.

Bulk earthworks are proposed to achieve the design levels across the site. The earthworks methodology and depth of cut and fill will affect the I_{ss} value, and subsequent y_s .

A specific site classification ('soil test') must be undertaken for each building on completion of earthworks, prior to building design and construction. It should be noted that the site classification will be significantly different within five years of fill placement compared to natural soils or older fill of similar nature (due to the application of 'cracked depth' in accordance with AS2870).

Based on the above information we have carried out a number of calculations using the estimated I_{ss} value and natural/fill soil profile scenarios to estimate possible y_s .

The range of results and the conditions for which those results were calculated is presented in Table 7 and Diagram 1 below.

Table 7: Estimated characteristic ground surface movements (y_s)

Assumed I_{ss} Value (Origin)	Estimated characteristic ground surface movement y_s (mm) across site				
	Existing Conditions	Natural surface (mm)			3 m Fill (Re- placed Residual)
		Cut 1 m	Cut to Rock & Fill 0.6 m (XW)	Cut to Rock & Fill 0.6 m (Residual Soil)	
5.2% (Residual Soil) 2.5% (XW Material)	65	25	40	70	140

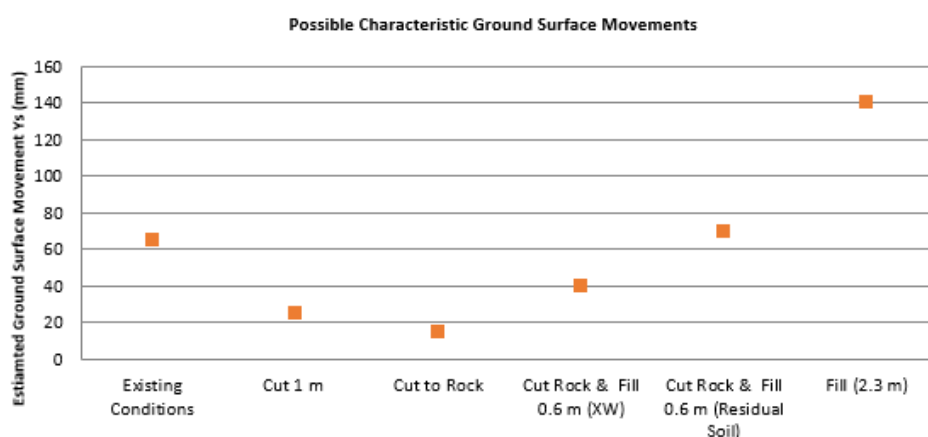


Diagram 1: Estimated characteristic ground surface movements

As the site soils are anticipated to be 'reactive', it will be necessary to ensure that 'usual good practice' is adopted for design and detailing if control of reactive ground movement is desired. Such practices should include the following:

- Site grades should be designed to readily shed water and prevent ponding around footings and other movement sensitive areas.

- Trees and shrubs should not be planted or be allowed to remain closer than their mature height to movement sensitive features.
- Soil moisture should not be allowed to change during or following construction.
- Buildings should include sealed ground surfaces around the perimeter, e.g. concrete or paving.
- The proposed buildings should be made as 'flexible' as possible to cope with reactive soil movements.
- Services beneath the buildings should be kept to a minimum and should be designed to be flexible (especially at building walls), and to prevent any leakage.

7.2.2 High Level Footings

Where ground slopes are less than 15°, building loads could be supported on footings penetrating at least 0.5 m into stiff (or stiffer) residual clays or future controlled Level 1 fill, with preliminary design allowable bearing pressures as follows:

- Stiff residual clays or Level 1 fill (95% Standard MDD (minimum)): 100 kPa
- Very stiff residual clay: 200 kPa
- Hard clays/extremely weathered rock: 400 kPa
- Medium strength (or stronger) rock: 600 kPa

7.2.3 Piled Foundations

Where ground slopes exceed 15° or where greater capacity is required footings should extend at least 0.5 m into medium strength (or stronger) rock. Bored piles are considered a suitable piling option where ground conditions dictate. Recommended design parameters that may be used for the pile design purposes are provided in Table 8.

Piles may also be required to manage differential settlement in areas where building footprints include shallow fill/cut and deep fill. Individual 'soil tests' will be required for each lot following earthworks, to confirm footing design.

Settlement of footings are expected to be in the order of 0.5 to 1% of the footing width or pile diameter.

Ultimate geotechnical capacities ($R_{d,ug}$) determined from the parameters provided in Table 8 must be multiplied by a geotechnical strength reduction factor (ϕ_g) in accordance with Section 4 of AS 2159-2009 Piling – Design and installation, to determine the design geotechnical strength ($R_{d,g}$). The geotechnical strength reduction factor should be determined in accordance with AS 2159-2009 and is expected to be in the range of 0.45 to 0.55. An upper bound value may be adopted if supervision and certification of pile construction is carried out by a geotechnical engineer.

The soils to a depth 1.5 times the pile diameter below the excavated surface should be ignored in the assessment of lateral capacity. The lateral capacity of piles may be determined using various methods, and which generally require an estimation of the Young's modulus (E_s) of the founding stratum.

Specialist piling contractors should be consulted with respect to selection of suitable pile types, detailed design of piles, the potential vibration effects caused by pile driving (if deemed suitable) and suitable dynamic pile testing methods and testing frequency. Table 8 shows representative values of ultimate (unfactored) geotechnical parameters for pile design. A sensitivity analysis should be undertaken to assess effects for the range of parameters.

Table 8: Recommended value for pile design.

Material type	Strength	Unit weight γ (kN/m ³)	C_u (kPa)	Young's Modulus, E_s (MPa)		Poisson's Ratio	Ultimate skin friction (kPa)	Ultimate base resistance (kPa)
				Drained, E'_s	Undrained, E_u			
Clay	Stiff	18	50	12	20	0.3 – 0.4	30	NR
	Very Stiff	19	100	15	25		50	
	Hard	20	200	30	50		80	
Basalt	Very Low Strength (or stronger)	21	-	150		0.2 – 0.3	100	3000

Notes:
Skin friction should be discounted for sections of pile shafts where liners are in place.
NR – Not recommended.

Footings must be cleaned prior to pouring concrete. Footing inspection by a suitably qualified RPEQ is recommended.

If any soil conditions encountered during footing construction are found to differ from those noted in the geotechnical investigation, Core should be notified immediately, and further assessment carried out to determine if changes to footing design are required. Inspection of footing excavations by a geotechnical engineer is recommended.

All footings should be founded such that they are not adversely affected by any adjacent excavations, batter slopes, trenches, or retaining walls that are not designed to support building loads. To minimise the potential for any adverse interaction effects, footings should be founded at least below a plane extending 1 m horizontally from the base of trenches/batter slopes/excavations/retaining walls, then rising up at 1V:1H. This requirement is illustrated in Diagram 2.

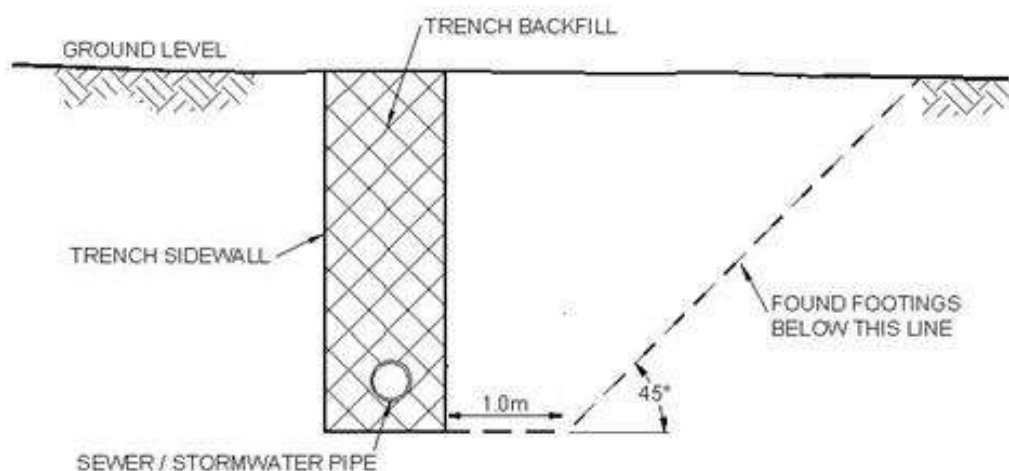


Diagram 2: Exclusion zones for base of footings

7.3 Drainage

Site drainage should be designed to readily remove surface water, minimise water infiltration into slopes, and to prevent ponding of water on subgrade areas during construction, and also adjacent to foundations, batters and driveways once construction is completed.

Roof water, including overflow from rainwater tanks, and water collected from hard surfaces (e.g. driveways) must be directed by pipes or lined channels to a legal point of discharge.

Shallow subsurface cut-off drains should be installed upslope of building areas in order to intercept and divert potential subsurface seepage. Diversion drains should be constructed above the crest of any cut or fill embankments.

Subsurface disposal of stormwater must not be undertaken.

7.4 General Foundation Maintenance

The CSIRO document Foundation maintenance and footing performance: A Homeowner's Guide (2003) is attached in Appendix F for your information. This advice should be considered in the construction and maintenance of the proposed development.

8. Preliminary Pavement Design (CBR) Parameters

Design parameters for pavements will depend on the subgrade materials present after earthworks and the type, depth and quality of any fill used to bring the site to design levels. If subgrade conditions exposed following earthworks differ from those encountered in the test pits, further subgrade evaluation (including further testing) should be undertaken during the construction stage.

The anticipated subgrades are expected to mainly comprise residual clay and extremely weathered materials, future fill (re-used site-won clay fill materials or imported fill) and medium strength or stronger basalt.

Laboratory test results indicate a soaked CBR of 2% for the existing residual clayey materials. Based on our experience with such materials, preliminary CBR design values are shown below in Table 9.

Table 9: Preliminary CBR Values

Origin	Laboratory CBR	Preliminary Design CBR
RESIDUAL SOIL	2	2
EXTREMELY WEATHERED BASALT	-	10
MEDIUM STRENGTH (or stronger) BASALT	-	15
FUTURE FILL	*	*
Notes: *To be confirmed following laboratory testing		

9. LIMITATIONS

Your attention is drawn to the document, *Limitations* which is attached in Appendix G.

SIGNATURES



Harrison Opie
BE(Civil)(Hons) MIEAust
Geotechnical Engineer



Walter Mastenbroek
MEngPr(Civil) MSc(Env) BSc (Hons) MIEAust CPEng NER RPEQ 16149
Director/Principal Geotechnical Engineer

Appendix A:

Figure 1 – Site Plan



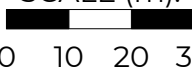
LEGEND

-  APPROXIMATE TEST PIT LOCATION
-  APPROXIMATE SLOPE ANGLE AND DIRECTION
-  APPROXIMATE SITE BOUNDARY



Projection: UTM
Datum: Australian GDA2020 - GRS 1980 / IUGG 1980

SCALE (m):



0 10 20 30

DRAWN	DS	CLIENT:	TUNNEY GROUP		
CHECKED	HO	PROJECT:	PROPOSED RESIDENTIAL SUBDIVISION 229 ROBSON HURSLEY ROAD, TORRINGTON		
DATE	14/05/2026	TITLE:	SITE PLAN		
SCALE	AS SHOWN	PROJECT NO:	J003223	FIGURE NO:	001
ORIGINAL SIZE	A3	REV:	0		

THIS DOCUMENT MUST BE READ IN CONJUNCTION WITH CORE DOCUMENT J003223-001-R-REV0.
DESIGN OVERLAY FROM DRAWING NO. MP-01 REVISION P.
BASE IMAGE FROM NEARMAP (IMAGE DATED 6 APRIL 2026).

Appendix B:

Results of Test Pits and Explanatory Notes

Client : Tunney Group
 Project : Proposed Subdivision
 Location : 229 Robson Hursley Rd, Torrington QLD 4350, Australia
 Job No : J003223

East : 390,795.00
 North : 6,951,794.00 56J
 Contractor : Jara Constructions & Civil
 Excavator : 20t Excavator
 Inclination : -90 deg

Sheet : 1 OF 1
 Logged : HO
 Logged Date : 14/05/2026
 Checked : SM
 Checked Date : 12/06/2026

METHOD	PENETRATION RESISTANCE TEST	WATER	DEPTH (meters)	DEPTH RL	SAMPLE OR FIELD TEST	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE DESCRIPTION	CONSISTENCY DENSITY	DCP TEST (blows per 100 mm)
EX	L	Groundwater not encountered	0.05				CI	Silty CLAY (TOPSOIL): medium plasticity, dark brown, with rootlets.	W	St	
			0.2		PP 500+kPa		CH	Silty CLAY (RESIDUAL SOIL): high plasticity, red brown, with roots.	W	VSt	
							CI	Silty CLAY (EXTREMELY WEATHERED BASALT): medium plasticity, pale brown with red brown.		H	
			1.0	1							Excavated to this depth before recommencing test
			2.0				BAS	BASALT: fine grained, pale brown, high strength, highly weathered, highly fractured, zones of extremely weathered 30 mm up to 50 mm.	D	HS	
			2.3								
							BAS	BASALT: fine grained, grey, high strength, moderately weathered, highly fractured, zones of extremely weathered 30mm up to 50mm.			
								TP1 Terminated at 3 m (Target Depth Backfilled)			

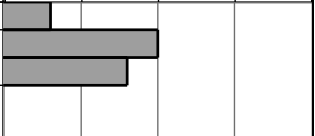
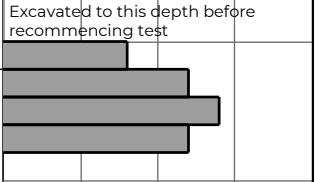


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Client : Tunney Group
 Project : Proposed Subdivision
 Location : 229 Robson Hursley Rd, Torrington QLD 4350, Australia
 Job No : J003223

East : 390,770.00
 North : 6,951,847.00 56J
 Contractor : Jara Constructions & Civil
 Excavator : 20t Excavator
 Inclination : -90 deg

Sheet : 1 OF 1
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METHOD	PENETRATION RESISTANCE TEST	WATER	DEPTH (meters)	DEPTH RL	SAMPLE OR FIELD TEST	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE DESCRIPTION	CONSISTENCY DENSITY	DCP TEST (blows per 100 mm)	
EX	L	Groundwater not encountered	0.1				CI	Silty CLAY (TOPSOIL): medium plasticity, brown, with rootlets, with cobbles.	w < PL	St		
			0.2				CI	Very stiff.		VSt		
			0.3		BDS Depth: 0.2-0.6m PP at 0.55m 600+ kPa		CH	CLAY (RESIDUAL SOIL): high plasticity, red brown, with cobbles.				
			0.6			CH	Very stiff to hard.	VSt-H				
	M		1.0				CI	Silty CLAY (EXTREMELY WEATHERED BASALT): medium plasticity, pale yellow and orange brown, with cobbles.			Excavated to this depth before recommencing test 	
			1.1									
					CI		Hard.	H				
			1.8									
	M-H		2.0				BAS	BASALT: fine grained, grey, medium strength, highly weathered, highly fractured with extremely weathered zones.	D	MS		
	H											
								TP2 Terminated at 3 m (Target Depth Backfilled)				



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 Project : Proposed Subdivision
 Location : 229 Robson Hursley Rd, Torrington QLD 4350, Australia
 Job No : J003223

East : 390,723.00
 North : 6,951,860.00 56J
 Contractor : Jara Constructions & Civil
 Excavator : 20t Excavator
 Inclination : -90 deg

Sheet : 1 OF 1
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METHOD	PENETRATION RESISTANCE TEST	WATER	DEPTH (meters)	DEPTH RL	SAMPLE OR FIELD TEST	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE DESCRIPTION	CONSISTENCY DENSITY	DCP TEST (blows per 100 mm)				
EX	L-M	Groundwater not encountered	0.1				CI	Silty CLAY (TOPSOIL): medium plasticity, brown, with rootlets, with cobbles.	w < PL	St					
			0.2			CI	Very stiff.	VSt							
	0.5		PP 600kPa Depth: 0.2-0.6m		CH	Silty CLAY (RESIDUAL SOIL): high plasticity, red brown, with cobbles.									
	0.6				CH	Hard.	H								
	M		1.0				CI	Silty CLAY (EXTREMELY WEATHERED BASALT): medium plasticity, red grey and orange brown, with cobbles.			Excavated to this depth before recommencing test				
			1.5												
			2.0												
	M-H		2.5				BAS	BASALT: fine grained, pale brown and grey, medium to high strength, highly weathered, highly fractured with extremely weathered zones.	D	MS-HS					
			H						BAS	very high strength, slightly weathered, with extremely weathered seams.		VHS			
								TP3 Refusal at 2.8 m (Practical Refusal Backfilled)							

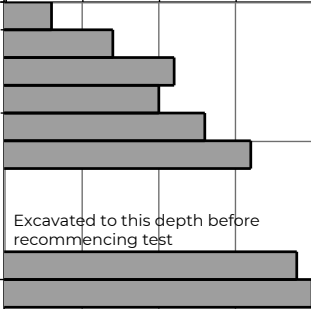


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 Project : Proposed Subdivision
 Location : 229 Robson Hursley Rd, Torrington QLD 4350, Australia
 Job No : J003223

East : 390,675.00
 North : 6,951,822.00 56J
 Contractor : Jara Constructions & Civil
 Excavator : 20t Excavator
 Inclination : -90 deg

Sheet : 1 OF 1
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					U50 Tube 0.2-0.5m		CH	CLAY (RESIDUAL SOIL): high plasticity, red brown.																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						
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			0.5		BDS Depth: 0.2-0.5		CI	Silty CLAY (EXTREMELY WEATHERED MATERIAL): medium plasticity, pale brown.																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						
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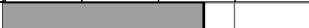


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 Project : Proposed Subdivision
 Location : 229 Robson Hursley Rd, Torrington QLD 4350, Australia
 Job No : J003223

East : 390,622.00
 North : 6,951,856.00 56J
 Contractor : Jara Constructions & Civil
 Excavator : 20t Excavator
 Inclination : -90 deg

Sheet : 1 OF 1
 Logged : HO
 Logged Date : 14/05/2026
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METHOD	PENETRATION RESISTANCE TEST	WATER	DEPTH (meters)	DEPTH RL	SAMPLE OR FIELD TEST	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE DESCRIPTION	CONSISTENCY DENSITY	DCP TEST (blows per 100 mm)	
EX	L	Groundwater not encountered	0.1				CI	Silty CLAY (TOPSOIL): medium plasticity, brown, with rootlets, with cobbles.	w < PL	St		
							CH	CLAY (RESIDUAL SOIL): high plasticity, brown to red brown.		H		
	0.5											
												
												
	M					CI	Sandy to silty CLAY (EXTREMELY WEATHERED BASALT): medium plasticity, pale brown, fine grained sand, with medium strength basalt gravel and cobbles.					
	1.0											
	1.1											
	M-H						BAS	BASALT: fine grained, grey and brown, high to very high strength, moderately weathered, highly fractured with extremely weathered seams up to 60mm.	D	HS-VHS		
			1.8				BAS	very high strength.		VHS		
			2.0				BAS	iron staining.		HS-VHS		
	H											
								TP5 Refusal at 2.4 m (Practical Refusal Backfilled)				



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 North : 6,951,855.00 56J
 Contractor : Jara Constructions & Civil
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METHOD	PENETRATION RESISTANCE TEST	WATER	DEPTH (meters)	DEPTH RL	SAMPLE OR FIELD TEST	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE DESCRIPTION	CONSISTENCY DENSITY	DCP TEST (blows per 100 mm)
EX	L	Groundwater not encountered	0.2				CI	Silty CLAY (TOPSOIL): medium plasticity, dark brown, with rootlets.	w < PL	St-VSt	
					PP 600+kPa		CH	Silty CLAY (RESIDUAL SOIL): high plasticity, grey brown.		VSt	
			0.4				CH	Hard.		H	
			0.6		PP 600+kPa		CI	Silty CLAY (EXTREMELY WEATHERED BASALT): medium plasticity, grey.			
			1.0								
			1.1				BAS	BASALT: fine grained, grey with white, high strength, highly weathered, highly fractured, fracture spacing up to 60 mm.	D	HS	
			1.6					highly fractured fracture spacing up to 150 mm.			
			2.0				BAS				
								TP6 Refusal at 2.1 m (Practical Refusal Backfilled)			

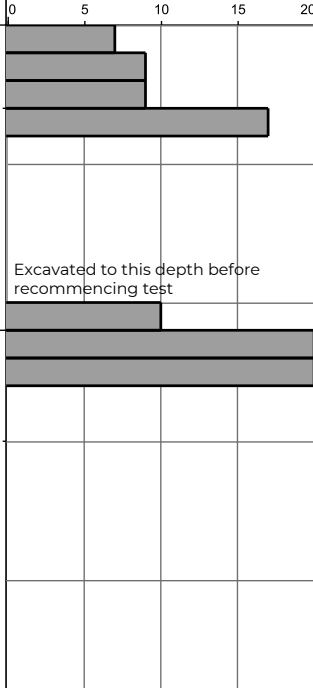


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 Project : Proposed Subdivision
 Location : 229 Robson Hursley Rd, Torrington QLD 4350, Australia
 Job No : J003223

East : 390,486.00
 North : 6,951,845.00 56J
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 Excavator : 20t Excavator
 Inclination : -90 deg

Sheet : 1 OF 1
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METHOD	PENETRATION RESISTANCE TEST	WATER	DEPTH (meters)	DEPTH RL	SAMPLE OR FIELD TEST	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE DESCRIPTION	CONSISTENCY DENSITY	DCP TEST (blows per 100 mm)
EX	L	Groundwater not encountered	0.15	 PP 450kPa		CI	Silty CLAY (TOPSOIL): medium plasticity, dark brown, with rootlets.	w < PL	VSt		
			CH			Silty CLAY (RESIDUAL SOIL): high plasticity, dark brown.					
			CH			Hard.					
	M		1.0			1.1	CI	Silty CLAY (EXTREMELY WEATHERED BASALT): medium plasticity, pale brown, with medium strength basalt gravel.	D	HS-VHS	Excavated to this depth before recommencing test
	M-H		1.5			BAS	BASALT: fine grained, grey and brown, high to very high strength, moderately weathered, highly fractured pieces up to 150mm, with extremely weathered seams.	VHS			
	H		2.0	BAS	very high strength.						
									TP7 Refusal at 2.4 m (Practical Refusal Backfilled)		

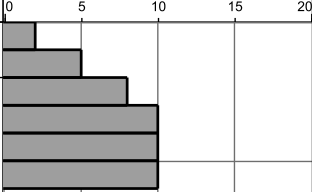
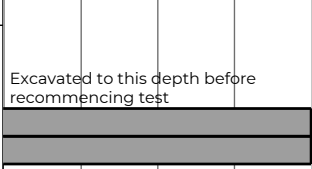


This report must be read in conjunction with accompanying notes and abbreviations. It has been prepared for geotechnical purposes only, without attempt to assess possible contamination. Any references to potential contamination are for information only and do not necessarily indicate the presence or absence of soil or groundwater contamination.

Client : Tunney Group
 Project : Proposed Subdivision
 Location : 229 Robson Hursley Rd, Torrington QLD 4350, Australia
 Job No : J003223

East : 390,518.00
 North : 6,951,899.00 56J
 Contractor : Jara Constructions & Civil
 Excavator : 20t Excavator
 Inclination : -90 deg

Sheet : 1 OF 1
 Logged : HO
 Logged Date : 14/05/2026
 Checked : SM
 Checked Date : 12/06/2026

METHOD	PENETRATION RESISTANCE TEST	WATER	DEPTH (meters)	DEPTH RL	SAMPLE OR FIELD TEST	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE DESCRIPTION	CONSISTENCY DENSITY	DCP TEST (blows per 100 mm)
EX	L	Groundwater not encountered	0.2				CI	Silty CLAY (TOPSOIL): medium plasticity, dark brown, with rootlets and roots.	w < PL	St-VSt	
			0.3				CI	Very stiff to hard.			
			0.7				CI	Silty CLAY (EXTREMELY WEATHERED BASALT): medium plasticity, pale grey. With fine to medium sand, with basalt gravels.			
	M		1.0				BAS	BASALT: fine grained, grey, medium to high strength, highly weathered, highly fractured with extremely weathered seams.	D	MS-HS	
	M-H		1.4				BAS	fracture spacing up to 100 mm.			
	H										
								TP8 Refusal at 1.8 m (Practical Refusal Backfilled)			



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DRILLING/EXCAVATION METHOD

AS	Auger Screwing	RD	Rotary blade or drag bit	NQ	Diamond Core - 47 mm
AD	Auger Drilling	RT	Rotary Tricone bit	NMLC	Diamond Core - 52 mm
*V	V - Bit	RAB	Rotary Air Blast	HQ	Diamond Core - 63 mm
T	TC - Bit, e.g. ADT	RC	Reverse Circulation	HMLC	Diamond Core – 63mm
HA	Hand Auger	PT	Push Tube	BH	Tractor Mounted Backhoe
ADH	Hollow Auger	CT	Cable Tool Rig	EX	Tracked Hydraulic Excavator
DTC	Diatubre Coring	JET	Jetting	EE	Existing Excavation
WB	Washbore or Bailer	NDD	Non-destructive digging	HAND	Excavated by Hand Methods

PENETRATION/EXCAVATION RESISTANCE

- L Low resistance** . Rapid penetration possible with little effort from the equipment used
- M Medium resistance**. Excavation possible at an acceptable rate with moderate effort from equipment used
- H High resistance to penetration/excavation**. Further penetration is possible at a slow rate
- R Refusal or Practical Refusal**. No further progress possible without the risk of damage or unacceptable wear to the digging implement or machine.

These assessments are subjective and are dependent on many factors including the equipment power, weight, condition of excavation or drilling tools, and the experience of the operator.

WATER



Water level shown at date



Water inflow



Partial water loss



Complete water loss

GROUNDWATER NOT OBSERVED The observation of groundwater whether present or not, was not possible due to drilling water, surface seepage or cave in of the borehole/test pit.

GROUND WATER NOT ENCOUNTERED The borehole/test pit was dry soon after excavation. However, groundwater could be present in less permeable strata. Inflow may have been observed had the borehole/test pit been left open for a longer period.

SAMPLING AND TESTING

SPT	Standard Penetration Test to AS1289.6.3.1-2004
4,7,11 N=18	4,7,11 = Blows per 150mm N = Blows per 300mm penetration following 150mm seating
30/80mm	Where practical refusal occurs, the blows and penetration for that interval are reported
RW	Penetration occurred under the rod weight only
HW	Penetration occurred under the hammer and rod weight only
HB	Hammer double bouncing on anvil
DS	Disturbed Sample
BDS	Bulk disturbed sample
G	Gas Sample
W	Water sample
FP	Field permeability test over section noted
FV	Field vane shear test expressed as uncorrected shear strength (sv = peak value)
PID	Photoionisation Detector reading in ppm
PM	Pressuremeter test over section noted
PP	Pocket penetrometer test expressed as instrument reading in kPa
U63	Thin walled tube sample - number indicates nominal sample diameter in millimetres
WPT	Water pressure tests
DCP	Dynamic cone penetration test
CPT	Dynamic cone penetration test
CPTu	Static cone penetration test with pore pressure (u) measurement

ROCK CORE RECOVERY

TCR = Total Core Recovery (%)

SCR = Solid Core Recovery (%)

RQD = Rock Quantity Designation (%)

$$= \frac{\text{Length of core recovered}}{\text{Length of core run}} \times 100$$

$$= \frac{\sum \text{Length of cylindrical core recovered}}{\text{Length of core run}} \times 100$$

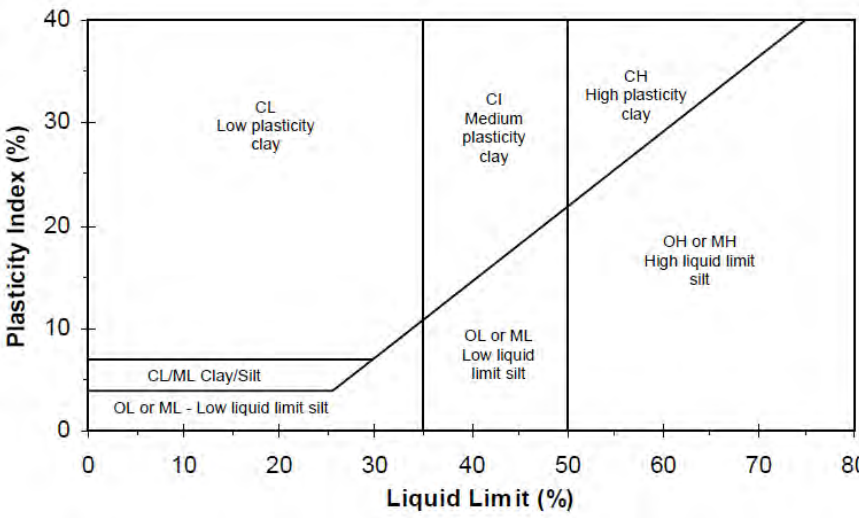
$$= \frac{\sum \text{Axial lengths of core} > 100 \text{ mm}}{\text{Length of core run}} \times 100$$

	FILL		CLAY (CL, CI, or CH)
	GRAVEL (GP or SW)		ORGANIC SOILS (OL or OH or Pt)
	SAND (SP or SW)		COBBLES or BOULDERS
	SILT (ML or MH)		

Combinations of these basic symbols may be used to indicate mixed materials such as sandy clay.

CLASSIFICATION AND INFERRED STRATIGRAPHY

Soil and Rock is classified and described in Reports of Boreholes and Test Pits using the preferred method given in AS 1726 - 2017. The material properties are assessed in the field by visual/tactile methods.

PARTICLE SIZE			PLASTIC PROPERTIES
Major	Sub Division	Particle Size	
Boulders		>200 mm	
Cobbles		63 - 200 mm	
Gravel	Coarse	20 - 63 mm	
Gravel	Medium	6.7 - 20 mm	
Gravel	Fine	2.36 - 6.7 mm	
Sand	Coarse	0.6 - 2.36 mm	
Sand	Medium	0.21 - 0.6 mm	
Sand	Fine	0.075 - 0.21 mm	
Silt		0.002 - 0.075 mm	
Clay		<0.002 mm	

MOISTURE CONDITION FOR COARSE GRAINED SOIL AS 1726 - 2017

Symbol	Term	Description
D	Dry	Non-cohesive and free running
M	Moist	Soil feels cool, darkened in colour, tends to stick together
W	Wet	Soil feels cool, darkened in colour, soil sticks together, free water forms when handling

MOISTURE CONDITION FOR FINE GRAINED SOIL AS1726 - 2017

Symbol	Term	Description
W<PL	Moist dry of liquid limit	Hard and friable or powdery
W = PL	Moist near plastic limit	Soils can be molded at a moisture condition approximately equal to the plastic limit
W > PL	Moist, wet of plastic limit	Soils usually weakened and free water forms on hands when handling
W = LL	Wet near plastic limit	
W > LL	Wet, wet of liquid limit	

CONSISTENCY TERMS FOR COHESIVE SOILS			RELATIVE DENSITY OF COARSE GRAINED SOILS			AS1726—2017
Symbol	Term	Undrained Shear Strength	Symbol	Term	Density Index %	SPT 'N' #
VS	Very Soft	0 to 12 kPa	VL	Very Loose	Less than 15	0 to 4
S	Soft	12 to 25 kPa	L	Loose	15 to 35	4 to 10
F	Firm	25 to 50 kPa	MD	Medium Dense	35 to 65	10 to 30
St	Stiff	50 to 100 kPa	D	Dense	65 to 85	30 to 50
VSt	Very Stiff	100 to 200 kPa	VD	Very Dense	Above 85	Above 50
H	Hard	Above 200 kPa	In the absence of test results, consistency and density may be assessed from correlations with the observed behaviour of the material.			

ROCK MATERIAL STRENGTH CLASSIFICATION				AS1726—2017
Symbol	Term	Uniaxial Compressive Strength (MPa)	Point Load Strength I_s (50) (MPa)	Field Guide
VL	Very Low Strength	0.6 to 2	0.03 to 0.1	Material crumbles under firm blows with sharp end of pick. Pieces up to 30 mm thick can be broken with finger pressure.
L	Low Strength	2 to 6	0.1 to 0.3	Easily scored with knife. Indentations 1 mm to 3 mm show in the specimen with firm blows of the pick point. A piece of core 150 mm by 50 mm may be broken by hand. Sharp edges of core are friable and break during handling.
M	Medium Strength	6 to 20	0.3 to 1	Readily scored with a knife. A piece of core 150 mm by 50 mm can be broken by hand with difficulty.
H	High Strength	20 to 60	1 to 3	A piece of core 150 mm by 50 mm cannot be broken by hand but can be broken by a pick with a single firm blow. Rock rings under hammer.
VH	Very High Strength	60 to 200	3 to 10	Hand specimen breaks with pick after more than one blow. Rock rings under hammer.
EH	Extremely High Strength	Above 200	Above 10	Specimen requires many blows with geological pick to break through intact material. Rock rings under hammer.

● = Diametral Point Load Test ▼ = Axial Point Load Test

CLASSIFICATION OF MATERIAL WEATHERING		AS1726—2017
Symbol	Term	Field Guide
RS	Residual Soil (<i>Note 1</i>)	Material is weathered to such an extent that it has soil properties. Mass structure and material texture and fabric of original rock are no longer visible but the soil has not been significantly transported.
XW	Extremely Weathered (<i>Note 1</i>)	Material is weathered to such an extent that it has soil properties. Mass structure and material texture and fabric of original rock are still visible.
HW	Highly Weathered (<i>Note 2</i>)	The whole rock mass is discoloured, usually by iron staining or beaching to the extent that the colour of the original rock is not recognizable. Rock strength is significantly changed by weathering. Some primary minerals have weathered to clay minerals. Porosity may be increased by leaching, or may be decreased due to deposition of weathering products in pores.
MH	Moderately Weathered (<i>Note 2</i>)	The whole of the rock material is discoloured, usually by iron staining or bleaching to the extent that the colour of the original rock is not recognizable, but shows little or no change in strength from fresh rock.
SW	Slightly Weathered	Rock is partially discoloured with staining or bleaching along joints but shows little or no change of strength from fresh rock.
FR	Fresh	Rock shows no signs of decomposition of individual minerals or colour change.
Note 1	The term 'Extremely Weathered rock' is misleading as the material has soil properties. The word 'rock' should be replaced with the name of the original rock of the word 'material', eg. Extremely Weathered granite or Extremely Weathered material.	
Note 2	Where it is not possible to distinguish between 'Highly Weathered' and 'Moderately Weathered' rock the term 'Distinctly Weathered' may be used.	

DEFECT TYPE/DESCRIPTION				DEFECT PROFILE		DEFECT ROUGHNESS	
				Symbol	Description	Symbol	Description
B	Bedding Parting	V	Vein	PL	Planar	SI	Slickenside
J	Joint	HB/DB	Handling/Drilling Break	St	Stepped	Sm	Smooth
EW	Extremely Weathered Seam	C	Contact	Un	Undulating	Ro	Rough
FZ	Fracture Zone	L	Cleavage	DEFECT INFILL DESCRIPTION		Vertical Boreholes - The dip (inclination from horizontal) for the defect is given. Inclined Boreholes - The inclination is measured as the acute angle to the core axis.	
CZ/S	Crushed Zone/Seam	X	Foliation	Symbol	Description		
IS	Infilled Seam	S	Schistosity	Cn	Clean: No visible coating		
SZ/S	Sheared Zone/Seam			Sn	Stain: Coated 1 to 3 mm		
				Vr	Veneer: < 1 mm		
				Ct	Coating: 1 to 3 mm		

Appendix C:

Laboratory Test Results

Material Test Report

Report Number: B-26-1116-1
Issue Number: 1
Date Issued: 09/06/2026
Client: Core Consultants Pty Ltd
 Unit 3/31 Londor Close, Hemmant Qld 4174
Contact: Harrison Opie
Project Number: B-26-1116
Project Name: J3223 - Proposed Sub-division - Reilly St, Torrington
Client Reference: J3223
Work Request: 22478
Sample Number: B-22478A
Date Sampled: 15/05/2026
Dates Tested: 19/05/2026 - 01/06/2026
Sampling Method: Sampled by Client - Tested as Received
The results apply to the sample as received
Preparation Method: AS 1289.1.1 - Sampling and Preparation of Soils
Site Selection: Selected by Client
Sample Location: TP2, Depth: 0.2 - 0.6m
Material: Clay
Material Source: Existing Materials



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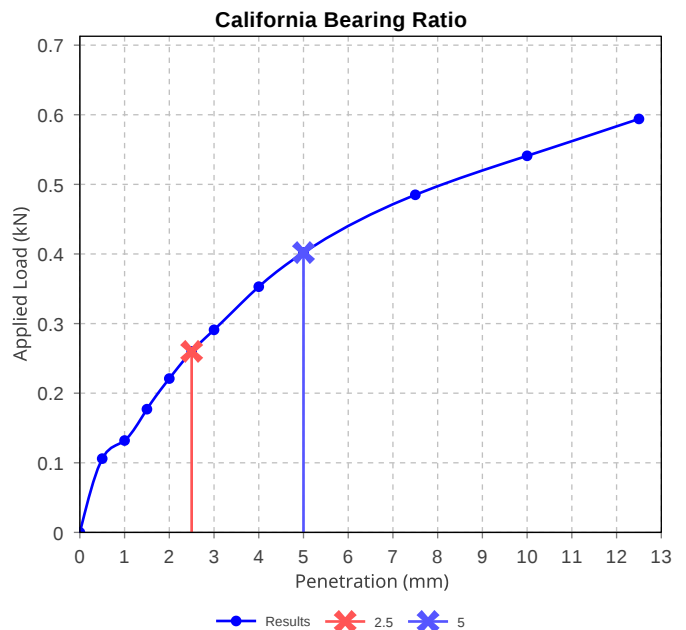
Approved Signatory: Torin Pegler
 Senior Soil Technician
 Laboratory Number: 2911

California Bearing Ratio (AS 1289 6.1.1 & 2.1.1)		Min	Max
CBR taken at	5 mm		
CBR %	2.0		
Method of Compactive Effort	Standard		
Method used to Determine MDD	AS1289 5.1.1		
Method used to Determine Plasticity	Visual		
Maximum Dry Density (t/m ³)	1.45		
Optimum Moisture Content (%)	29.0		
Laboratory Density Ratio (%)	95.0		
Laboratory Moisture Ratio (%)	99.0		
Dry Density after Soaking (t/m ³)	1.28		
Field Moisture Content (%)	26.7		
Moisture Content at Placement (%)	28.7		
Moisture Content Top 30mm (%)	43.2		
Moisture Content Rest of Sample (%)	35.8		
Mass Surcharge (kg)	4.5		
Soaking Period (days)	4		
Curing Hours (h)	210.8		
Swell (%)	8.0		
Oversize Material (mm)	19		
Oversize Material Included	Excluded		
Oversize Material (%)	0.0		

Atterberg Limit (AS1289 3.1.2 & 3.2.1 & 3.3.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	71		
Plastic Limit (%)	25		
Plasticity Index (%)	46		

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	20.0		
Cracking Crumbling Curling	Curling		

Emerson Class Number of a Soil (AS 1289.3.8.1)		Min	Max
Emerson Class	4 *		
Soil Description	Brown CLAY		
Nature of Water	Distilled		
* Mineral Present	Carbonate		



Material Test Report

Report Number: B-26-1116-1
Issue Number: 1
Date Issued: 09/06/2026
Client: Core Consultants Pty Ltd
Unit 3/31 Londor Close, Hemmant Qld 4174
Contact: Harrison Opie
Project Number: B-26-1116
Project Name: J3223 - Proposed Sub-division - Reilly St, Torrington
Client Reference: J3223
Work Request: 22478
Sample Number: B-22478B
Date Sampled: 15/05/2026
Dates Tested: 19/05/2026 - 22/05/2026
Sampling Method: Sampled by Client - Tested as Received
The results apply to the sample as received
Preparation Method: AS 1289.1.1 - Sampling and Preparation of Soils
Site Selection: Selected by Client
Sample Location: TP4, Depth: 0.2 - 0.5m
Material: Clay
Material Source: Existing Materials



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Phone: (07) 3284 8766
Email: torinme@hotmail.com



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Approved Signatory: Torin Pegler
Senior Soil Technician
Laboratory Number: 2911

Emerson Class Number of a Soil (AS 1289.3.8.1)		Min	Max
Emerson Class	4 *		
Soil Description	Brown Sandy CLAY		
Nature of Water	Distilled		
* Mineral Present	Carbonate		

Material Test Report

Report Number: B-26-1116-1
Issue Number: 1
Date Issued: 09/06/2026
Client: Core Consultants Pty Ltd
 Unit 3/31 Londor Close, Hemmant Qld 4174
Contact: Harrison Opie
Project Number: B-26-1116
Project Name: J3223 - Proposed Sub-division - Reilly St, Torrington
Client Reference: J3223
Work Request: 22478
Sample Number: B-22478C
Date Sampled: 15/05/2026
Dates Tested: 19/05/2026 - 01/06/2026
Sampling Method: Sampled by Client - Tested as Received
The results apply to the sample as received
Preparation Method: AS 1289.1.1 - Sampling and Preparation of Soils
Site Selection: Selected by Client
Sample Location: TP4, Depth: 0.2 - 0.5m
Material: Clay
Material Source: Existing Materials



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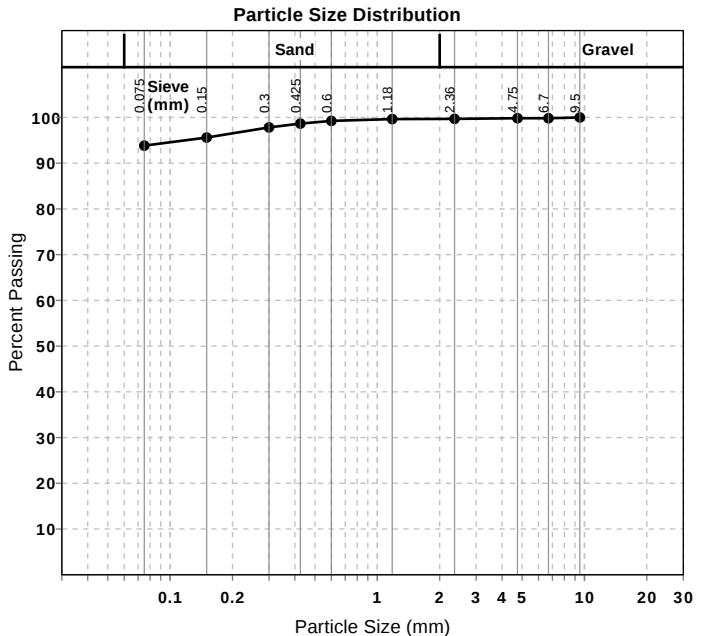
[Signature]

Approved Signatory: Torin Pegler
 Senior Soil Technician
 Laboratory Number: 2911

Particle Size Distribution (AS1289 3.6.1)		
Sieve	Passed %	Passing Limits
9.5 mm	100	
6.7 mm	100	
4.75 mm	100	
2.36 mm	100	
1.18 mm	100	
0.6 mm	99	
0.425 mm	99	
0.3 mm	98	
0.15 mm	96	
0.075 mm	94	

Atterberg Limit (AS1289 3.1.2 & 3.2.1 & 3.3.1 & Q252)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Passing 0.425 (%)	99		
Liquid Limit (%)	80		
Plastic Limit (%)	24		
Plasticity Index (%)	56		
Weighted Plasticity Index (%)	5525		

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	22.5		
Weighted Linear Shrinkage (%)	2210		
Cracking Crumbling Curling	Cracking & Curling		



Material Test Report

Report Number: B-26-1116-1
Issue Number: 1
Date Issued: 09/06/2026
Client: Core Consultants Pty Ltd
 Unit 3/31 Londor Close, Hemmant Qld 4174
Contact: Harrison Opie
Project Number: B-26-1116
Project Name: J3223 - Proposed Sub-division - Reilly St, Torrington
Client Reference: J3223
Work Request: 22478
Sample Number: B-22478D
Date Sampled: 15/05/2026
Dates Tested: 19/05/2026 - 01/06/2026
Sampling Method: Sampled by Client - Tested as Received
The results apply to the sample as received
Preparation Method: AS 1289.1.1 - Sampling and Preparation of Soils
Site Selection: Selected by Client
Sample Location: TP7, Depth: 0.15 - 0.4m
Material: Clay
Material Source: Existing Materials



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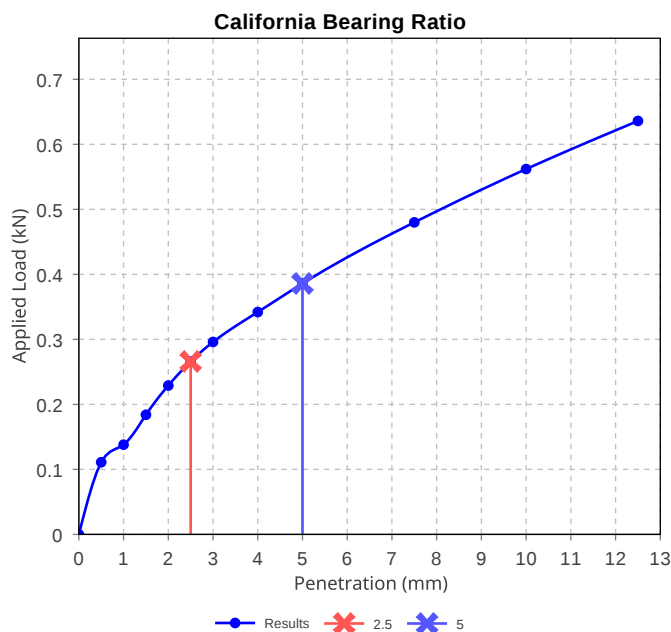
Approved Signatory: Torin Pegler
 Senior Soil Technician
 Laboratory Number: 2911

California Bearing Ratio (AS 1289 6.1.1 & 2.1.1)		Min	Max
CBR taken at	2.5 mm		
CBR %	2.0		
Method of Compactive Effort	Standard		
Method used to Determine MDD	AS 1289 5.1.1 & 2.1.1		
Method used to Determine Plasticity	Visual		
Maximum Dry Density (t/m ³)	1.37		
Optimum Moisture Content (%)	24.0		
Laboratory Density Ratio (%)	95.0		
Laboratory Moisture Ratio (%)	100.0		
Dry Density after Soaking (t/m ³)	1.21		
Field Moisture Content (%)	21.6		
Moisture Content at Placement (%)	24.0		
Moisture Content Top 30mm (%)	41.3		
Moisture Content Rest of Sample (%)	36.2		
Mass Surcharge (kg)	4.5		
Soaking Period (days)	4		
Curing Hours (h)	210.7		
Swell (%)	8.0		
Oversize Material (mm)	19		
Oversize Material Included	Excluded		
Oversize Material (%)	0.0		

Atterberg Limit (AS1289 3.1.2 & 3.2.1 & 3.3.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	69		
Plastic Limit (%)	21		
Plasticity Index (%)	48		

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	17.0		
Cracking Crumbling Curling	Cracking & Curling		

Emerson Class Number of a Soil (AS 1289.3.8.1)		Min	Max
Emerson Class	4 *		
Soil Description	Dark Brown sandy CLAY		
Nature of Water	Distilled		
* Mineral Present	Carbonate		



Material Test Report

Report Number: B-26-1116-1
Issue Number: 1
Date Issued: 09/06/2026
Client: Core Consultants Pty Ltd
Unit 3/31 Londor Close, Hemmant Qld 4174
Contact: Harrison Opie
Project Number: B-26-1116
Project Name: J3223 - Proposed Sub-division - Reilly St, Torrington
Client Reference: J3223
Work Request: 22478
Sample Number: B-22478E
Date Sampled: 15/05/2026
Dates Tested: 19/05/2026 - 01/06/2026
Sampling Method: Sampled by Client - Tested as Received
The results apply to the sample as received
Preparation Method: AS 1289.1.1 - Sampling and Preparation of Soils
Site Selection: Selected by Client
Sample Location: TP7, Depth: 0.15 - 0.4m
Material: Clay
Material Source: Existing Materials



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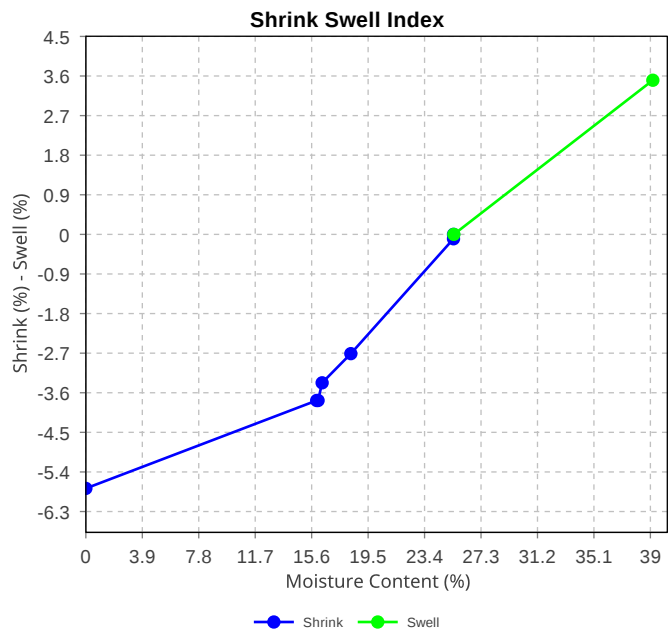


Approved Signatory: Torin Pegler
Senior Soil Technician
Laboratory Number: 2911

Shrink Swell Index (AS 1289 7.1.1 & 2.1.1)		Min	Max
Iss (%)*	4.2		
Visual Description	Silty Clay		
* Shrink Swell Index (Iss) reported as the percentage vertical strain per pF change in suction.			

Core Shrinkage Test	
Total Shrinkage Strain (%)	5.8
Extent of Cracking	Minor
Extent of Crumbling	None
Moisture Content (%)	25.4
Initial Bulk Density (t/m ³)	1.779

Swell Test	
Initial Pocket Penetrometer (kPa)*	
Final Pocket Penetrometer (kPa)*	
Initial Moisture Content (%)	25.4
Final Moisture Content (%)	39.2
Total Swell Strain (%)	3.5
Initial Bulk Density (t/m ³)	1.658
* scope of accreditation does not cover the performance of pocket penetrometer readings.	



Material Test Report

Report Number: B-26-1116-1
Issue Number: 1
Date Issued: 09/06/2026
Client: Core Consultants Pty Ltd
Unit 3/31 Londor Close, Hemmant Qld 4174
Contact: Harrison Opie
Project Number: B-26-1116
Project Name: J3223 - Proposed Sub-division - Reilly St, Torrington
Client Reference: J3223
Work Request: 22478
Date Sampled: 15/05/2026
Dates Tested: 19/05/2026 - 19/05/2026
Sampling Method: Sampled by Client - Tested as Received
The results apply to the sample as received
Preparation Method: AS 1289.1.1 - Sampling and Preparation of Soils
Location: J3223 - Proposed Sub-division - Reilly St, Torrington
Material: Clay
Material Source: Existing Materials



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Approved Signatory: Torin Pegler
Senior Soil Technician
Laboratory Number: 2911

Moisture Content AS 1289 2.1.1

Sample Number	Sample Location	Moisture Content (%)	Min	Max	Material
B-22478A	TP2, Depth: 0.2 - 0.6m	20.8 %	**	**	Clay
B-22478D	TP7, Depth: 0.15 - 0.4m	21.5 %	**	**	Clay

Appendix D:

Qualitative Risk Analysis Matrix (Australian Geomechanics Vol 42 No 1 March 2007)

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007
APPENDIX C: LANDSLIDE RISK ASSESSMENT
QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY

QUALITATIVE MEASURES OF LIKELIHOOD

Approximate Annual Probability		Implied Indicative Landslide Recurrence Interval		Description	Descriptor	Level
Indicative Value	Notional Boundary					
10 ⁻¹	5x10 ⁻²	10 years	20 years	The event is expected to occur over the design life.	ALMOST CERTAIN	A
10 ⁻²		100 years		The event will probably occur under adverse conditions over the design life.	LIKELY	B
10 ⁻³	5x10 ⁻³	1000 years	200 years	The event could occur under adverse conditions over the design life.	POSSIBLE	C
10 ⁻⁴	5x10 ⁻⁴	10,000 years	2000 years	The event might occur under very adverse circumstances over the design life.	UNLIKELY	D
10 ⁻⁵	5x10 ⁻⁵	100,000 years	20,000 years	The event is conceivable but only under exceptional circumstances over the design life.	RARE	E
10 ⁻⁶	5x10 ⁻⁶	1,000,000 years	200,000 years	The event is inconceivable or fanciful over the design life.	BARELY CREDIBLE	F

Note: (1) The table should be used from left to right; use Approximate Annual Probability or Description to assign Descriptor, not *vice versa*.

QUALITATIVE MEASURES OF CONSEQUENCES TO PROPERTY

Approximate Cost of Damage		Description	Descriptor	Level
Indicative Value	Notional Boundary			
200%	100%	Structure(s) completely destroyed and/or large scale damage requiring major engineering works for stabilisation. Could cause at least one adjacent property major consequence damage.	CATASTROPHIC	1
60%		Extensive damage to most of structure, and/or extending beyond site boundaries requiring significant stabilisation works. Could cause at least one adjacent property medium consequence damage.	MAJOR	2
20%	40%	Moderate damage to some of structure, and/or significant part of site requiring large stabilisation works. Could cause at least one adjacent property minor consequence damage.	MEDIUM	3
5%	10%	Limited damage to part of structure, and/or part of site requiring some reinstatement stabilisation works.	MINOR	4
0.5%	1%	Little damage. (Note for high probability event (Almost Certain), this category may be subdivided at a notional boundary of 0.1%. See Risk Matrix.)	INSIGNIFICANT	5

- Notes:** (2) The Approximate Cost of Damage is expressed as a percentage of market value, being the cost of the improved value of the unaffected property which includes the land plus the unaffected structures.
- (3) The Approximate Cost is to be an estimate of the direct cost of the damage, such as the cost of reinstatement of the damaged portion of the property (land plus structures), stabilisation works required to render the site to tolerable risk level for the landslide which has occurred and professional design fees, and consequential costs such as legal fees, temporary accommodation. It does not include additional stabilisation works to address other landslides which may affect the property.
- (4) The table should be used from left to right; use Approximate Cost of Damage or Description to assign Descriptor, not *vice versa*

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX C: – QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY (CONTINUED)

QUALITATIVE RISK ANALYSIS MATRIX – LEVEL OF RISK TO PROPERTY

LIKELIHOOD		CONSEQUENCES TO PROPERTY (With Indicative Approximate Cost of Damage)				
	Indicative Value of Approximate Annual Probability	1: CATASTROPHIC 200%	2: MAJOR 60%	3: MEDIUM 20%	4: MINOR 5%	5: INSIGNIFICANT 0.5%
A – ALMOST CERTAIN	10 ⁻¹	VH	VH	VH	H	M or L (5)
B – LIKELY	10 ⁻²	VH	VH	H	M	L
C – POSSIBLE	10 ⁻³	VH	H	M	M	VL
D – UNLIKELY	10 ⁻⁴	H	M	L	L	VL
E – RARE	10 ⁻⁵	M	L	L	VL	VL
F – BARELY CREDIBLE	10 ⁻⁶	L	VL	VL	VL	VL

Notes: (5) For Cell A5, may be subdivided such that a consequence of less than 0.1% is Low Risk.

(6) When considering a risk assessment it must be clearly stated whether it is for existing conditions or with risk control measures which may not be implemented at the current time.

RISK LEVEL IMPLICATIONS

Risk Level		Example Implications (7)
VH	VERY HIGH RISK	Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of the property.
H	HIGH RISK	Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.
M	MODERATE RISK	May be tolerated in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.
L	LOW RISK	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance is required.
VL	VERY LOW RISK	Acceptable. Manage by normal slope maintenance procedures.

Note: (7) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide.

Appendix E:

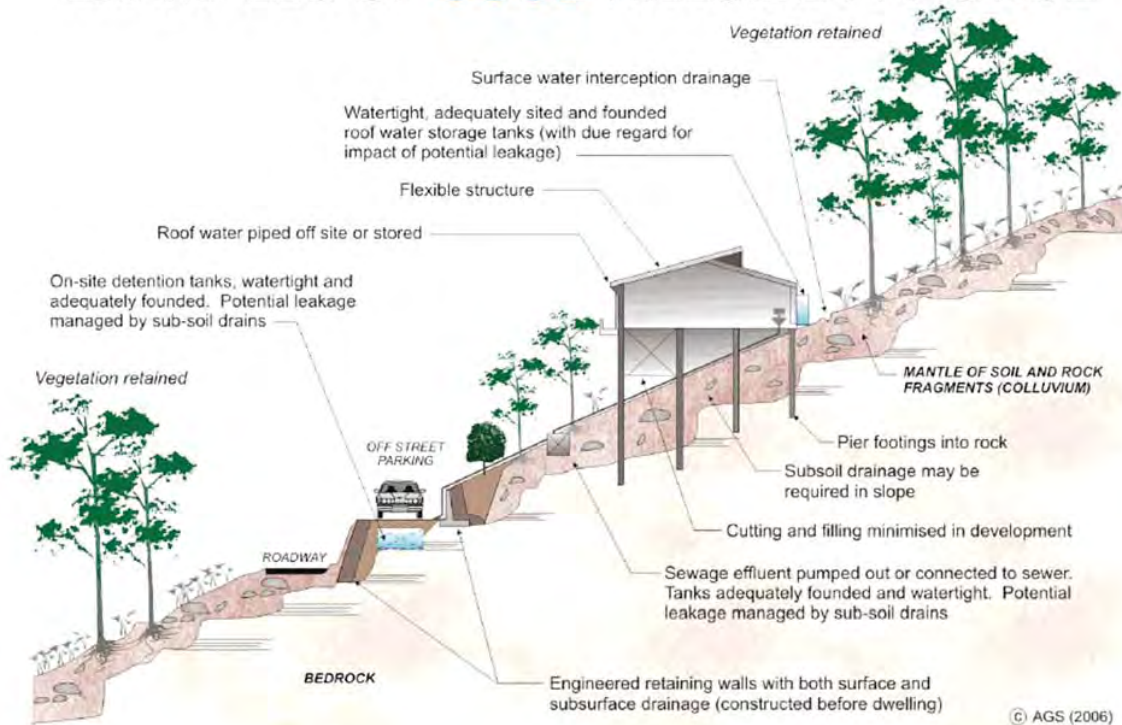
General Guidelines for Hillside Development

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

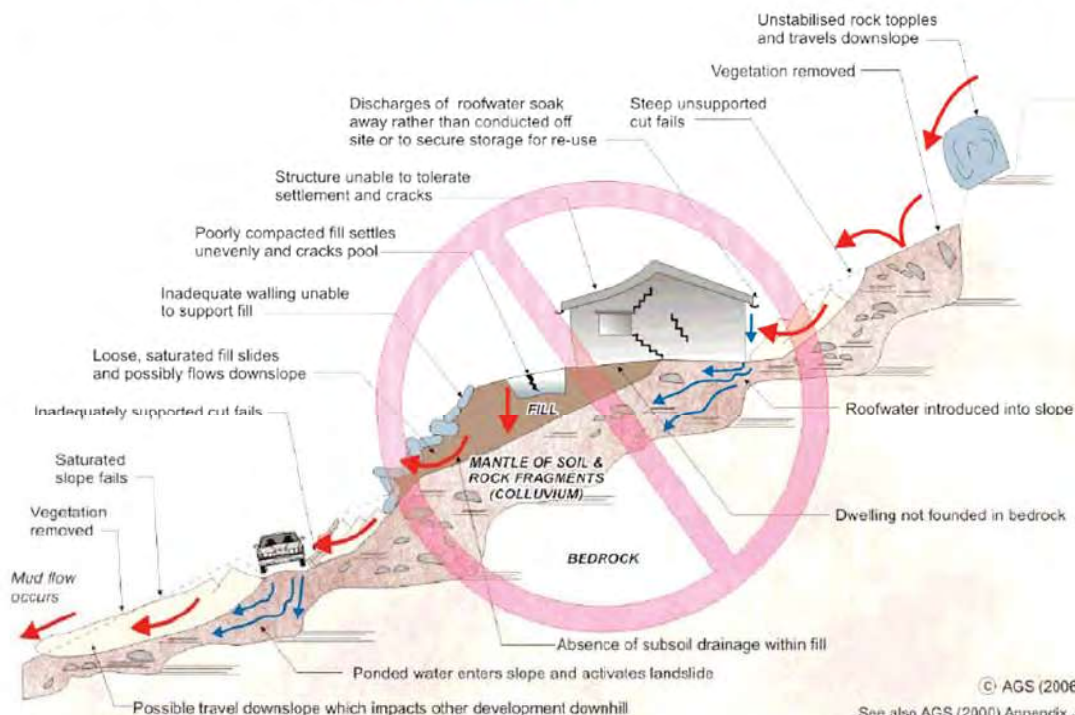
APPENDIX G - SOME GUIDELINES FOR HILLSIDE CONSTRUCTION

ADVICE		GOOD ENGINEERING PRACTICE	POOR ENGINEERING PRACTICE
GEOTECHNICAL ASSESSMENT		Obtain advice from a qualified, experienced geotechnical practitioner at early stage of planning and before site works.	Prepare detailed plan and start site works before geotechnical advice.
PLANNING			
SITE PLANNING		Having obtained geotechnical advice, plan the development with the risk arising from the identified hazards and consequences in mind.	Plan development without regard for the Risk.
DESIGN AND CONSTRUCTION			
HOUSE DESIGN		Use flexible structures which incorporate properly designed brickwork, timber or steel frames, timber or panel cladding. Consider use of split levels. Use decks for recreational areas where appropriate.	Floor plans which require extensive cutting and filling. Movement intolerant structures.
SITE CLEARING		Retain natural vegetation wherever practicable.	Indiscriminately clear the site.
ACCESS & DRIVEWAYS		Satisfy requirements below for cuts, fills, retaining walls and drainage. Council specifications for grades may need to be modified. Driveways and parking areas may need to be fully supported on piers.	Excavate and fill for site access before geotechnical advice.
EARTHWORKS		Retain natural contours wherever possible.	Indiscriminatory bulk earthworks.
CUTS		Minimise depth. Support with engineered retaining walls or batter to appropriate slope. Provide drainage measures and erosion control.	Large scale cuts and benching. Unsupported cuts. Ignore drainage requirements
FILLS		Minimise height. Strip vegetation and topsoil and key into natural slopes prior to filling. Use clean fill materials and compact to engineering standards. Batter to appropriate slope or support with engineered retaining wall. Provide surface drainage and appropriate subsurface drainage.	Loose or poorly compacted fill, which if it fails, may flow a considerable distance including onto property below. Block natural drainage lines. Fill over existing vegetation and topsoil. Include stumps, trees, vegetation, topsoil, boulders, building rubble etc in fill.
ROCK OUTCROPS & BOULDERS		Remove or stabilise boulders which may have unacceptable risk. Support rock faces where necessary.	Disturb or undercut detached blocks or boulders.
RETAINING WALLS		Engineer design to resist applied soil and water forces. Found on rock where practicable. Provide subsurface drainage within wall backfill and surface drainage on slope above. Construct wall as soon as possible after cut/fill operation.	Construct a structurally inadequate wall such as sandstone flagging, brick or unreinforced blockwork. Lack of subsurface drains and weepholes.
FOOTINGS		Found within rock where practicable. Use rows of piers or strip footings oriented up and down slope. Design for lateral creep pressures if necessary. Backfill footing excavations to exclude ingress of surface water.	Found on topsoil, loose fill, detached boulders or undercut cliffs.
SWIMMING POOLS		Engineer designed. Support on piers to rock where practicable. Provide with under-drainage and gravity drain outlet where practicable. Design for high soil pressures which may develop on uphill side whilst there may be little or no lateral support on downhill side.	
DRAINAGE			
SURFACE		Provide at tops of cut and fill slopes. Discharge to street drainage or natural water courses. Provide general falls to prevent blockage by siltation and incorporate silt traps. Line to minimise infiltration and make flexible where possible. Special structures to dissipate energy at changes of slope and/or direction.	Discharge at top of fills and cuts. Allow water to pond on bench areas.
SUBSURFACE		Provide filter around subsurface drain. Provide drain behind retaining walls. Use flexible pipelines with access for maintenance. Prevent inflow of surface water.	Discharge roof runoff into absorption trenches.
SEPTIC & SULLAGE		Usually requires pump-out or mains sewer systems; absorption trenches may be possible in some areas if risk is acceptable. Storage tanks should be water-tight and adequately founded.	Discharge sullage directly onto and into slopes. Use absorption trenches without consideration of landslide risk.
EROSION CONTROL & LANDSCAPING		Control erosion as this may lead to instability. Revegetate cleared area.	Failure to observe earthworks and drainage recommendations when landscaping.
DRAWINGS AND SITE VISITS DURING CONSTRUCTION			
DRAWINGS		Building Application drawings should be viewed by geotechnical consultant	
SITE VISITS		Site Visits by consultant may be appropriate during construction/	
INSPECTION AND MAINTENANCE BY OWNER			
OWNER'S RESPONSIBILITY		Clean drainage systems; repair broken joints in drains and leaks in supply pipes. Where structural distress is evident see advice. If seepage observed, determine causes or seek advice on consequences.	

EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE



Appendix F:

CSIRO Foundation Maintenance Guidelines

Foundation Maintenance and Footing Performance: A Homeowner's Guide



PUBLISHING

BTF 18-2011
replaces
Information
Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870-2011, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction

There are two types of settlement that occur as a result of construction:

- Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion

All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Saturation

This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume, particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.

In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

GENERAL DEFINITIONS OF SITE CLASSES

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites, which may experience only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which may experience moderate ground movement from moisture changes
H1	Highly reactive clay sites, which may experience high ground movement from moisture changes
H2	Highly reactive clay sites, which may experience very high ground movement from moisture changes
E	Extremely reactive sites, which may experience extreme ground movement from moisture changes

Notes

1. Where controlled fill has been used, the site may be classified A to E according to the type of fill used.
2. Filled sites. Class P is used for sites which include soft fills, such as clay or silt or loose sands; landslide; mine subsidence; collapsing soils; soil subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise.
3. Where deep-seated moisture changes exist on sites at depths of 3 m or greater, further classification is needed for Classes M to E (M-D, H1-D, H2-D and E-D).

Tree root growth

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Swelling gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpend).

Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

Seasonal swelling/shrinkage in clay

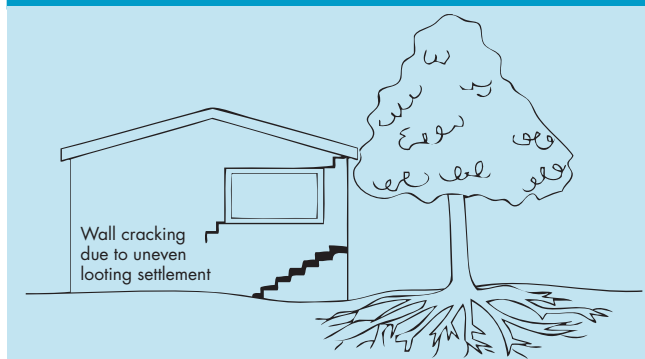
Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.

As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the

Trees can cause shrinkage and damage



external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself

Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures

Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation causes a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures

Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem. Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.

- Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870-2011.

AS 2870-2011 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/Cure

Plumbing

Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage

In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

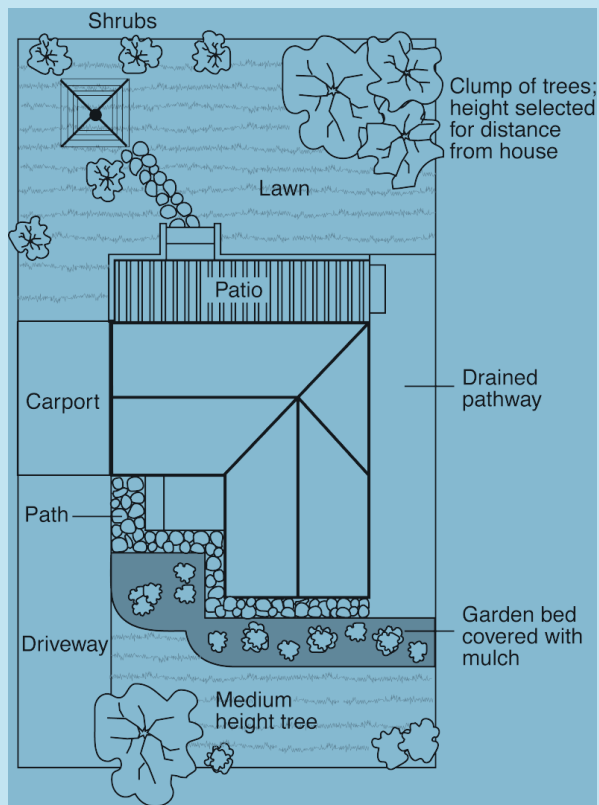
Protection of the building perimeter

It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving should

CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS		
Description of typical damage and required repair	Approximate crack width limit (see Note 3)	Damage category
Hairline cracks	<0.1 mm	0
Fine cracks which do not need repair	<1 mm	1
Cracks noticeable but easily filled. Doors and windows stick slightly.	<5 mm	2
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired.	5–15 mm (or a number of cracks 3 mm or more in one group)	3
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted.	15–25 mm but also depends on number of cracks	4

Gardens for a reactive site



extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

The information in this and other issues in the series was derived from various sources and was believed to be correct when published.

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Appendix G:

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Limitations

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FOUNDATION MAINTENANCE AND FOOTING PERFORMANCE

Preventing soil-related building movement

This Building Technology Resource is designed as a homeowner's guide on the causes of soil-related building movement, and suggested methods to prevent resultant cracking.

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the home owner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement. Generally soil classification is provided by a geotechnical report.

SOIL TYPES

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. Table 1 below is a reproduction of Table 2.1 from Australian Standard AS 2870-2011, Residential slabs and footings.

CAUSES OF MOVEMENT

SETTLEMENT DUE TO CONSTRUCTION

There are two types of settlement that occur as a result of construction:

- ▶ Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- ▶ Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction but has been known to take many years in exceptional cases.

These problems may be the province of the builder and should be taken into consideration as part of the preparation of the site for construction.

EROSION

All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

SATURATION

This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume,

particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

SEASONAL SWELLING AND SHRINKAGE OF SOIL

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below, from AS 2870). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

SHEAR FAILURE

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- ▶ Significant load increase.
- ▶ Reduction of lateral support of the soil under the footing due to erosion or excavation.

In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

TREE ROOT GROWTH

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- ▶ Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.

TABLE 1. GENERAL DEFINITIONS OF SITE CLASSES.

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites, which may experience only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which may experience moderate ground movement from moisture changes
H1	Highly reactive clay sites, which may experience high ground movement from moisture changes
H2	Highly reactive clay sites, which may experience very high ground movement from moisture changes
E	Extremely reactive sites, which may experience extreme ground movement from moisture changes

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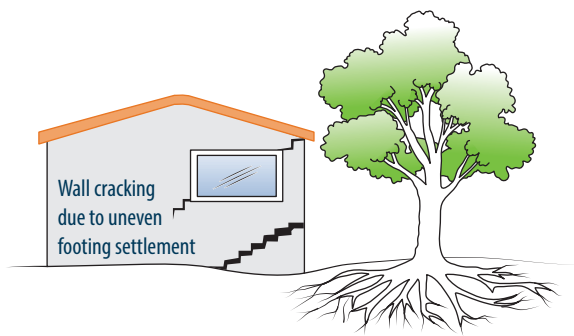


FIGURE 1 Trees can cause shrinkage and damage.

- ▶ Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

UNEVENNESS OF MOVEMENT

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- ▶ Differing compaction of foundation soil prior to construction.
- ▶ Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior through absorption. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Shrinkage usually begins on the side of the building where the sun's heat is greatest.

EFFECTS OF UNEVEN SOIL MOVEMENT ON STRUCTURES

EROSION AND SATURATION

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- ▶ Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- ▶ Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpend).

Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

SEASONAL SWELLING/SHRINKAGE IN CLAY

Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers

and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.

As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the external footings. The doming is accentuated, and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry, and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

MOVEMENT CAUSED BY TREE ROOTS

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

COMPLICATIONS CAUSED BY THE STRUCTURE ITSELF

Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

EFFECTS ON FULL MASONRY STRUCTURES

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also

exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

EFFECTS ON FRAMED STRUCTURES

Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation causes a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

EFFECTS ON BRICK VENEER STRUCTURES

Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

WATER SERVICE AND DRAINAGE

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem. Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- ▶ Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.
- ▶ Corroded guttering or downpipes can spill water to ground.
- ▶ Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

SERIOUSNESS OF CRACKING

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. Table 2 below is a reproduction of Table C1 of AS 2870-2011.

AS 2870-2011 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

PREVENTION AND CURE

PLUMBING

Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

GROUND DRAINAGE

In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject may be regarded as an area for an expert consultant.

PROTECTION OF THE BUILDING PERIMETER

It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving should extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill.

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

CONDENSATION

In buildings with a subfloor void, such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

TABLE 2. CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS.

Description of typical damage and required repair	Approximate crack width limit	Damage category
Hairline cracks	<0.1 mm	0 – Negligible
Fine cracks which do not need repair	<1 mm	1 – Very Slight
Cracks noticeable but easily filled. Doors and windows stick slightly.	<5 mm	2 – Slight
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired.	5–15 mm (or a number of cracks 3 mm or more in one group)	3 – Moderate
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted.	15–25 mm but also depends on number of cracks	4 – Severe

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Warning: Although this Building Technology Resource deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

- ▶ Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- ▶ High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders, and mould.
- ▶ Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

THE GARDEN

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

EXISTING TREES

Existing trees may cause problems with the upheaval of footings by their roots, or shrinkage from soil drying. If the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. Soil drying is a more complex issue and professional advice may be required before considering the removal or relocation of the tree.

INFORMATION ON TREES, PLANTS AND SHRUBS

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information.

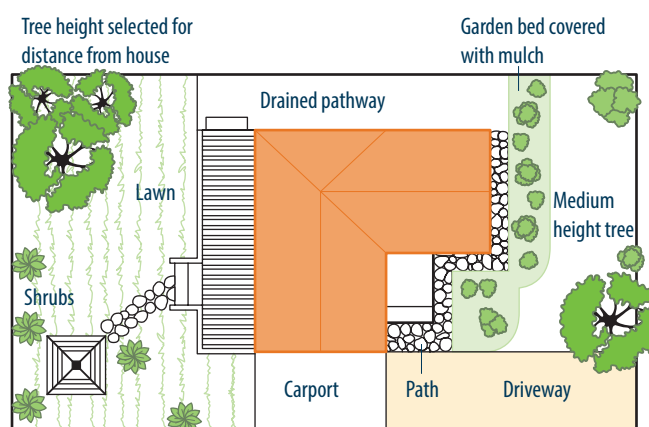


FIGURE 2 Gardens for a reactive site.

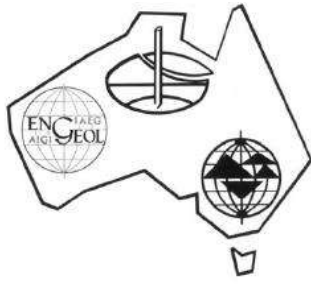
EXCAVATION

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

REMEDIATION

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the home owner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.



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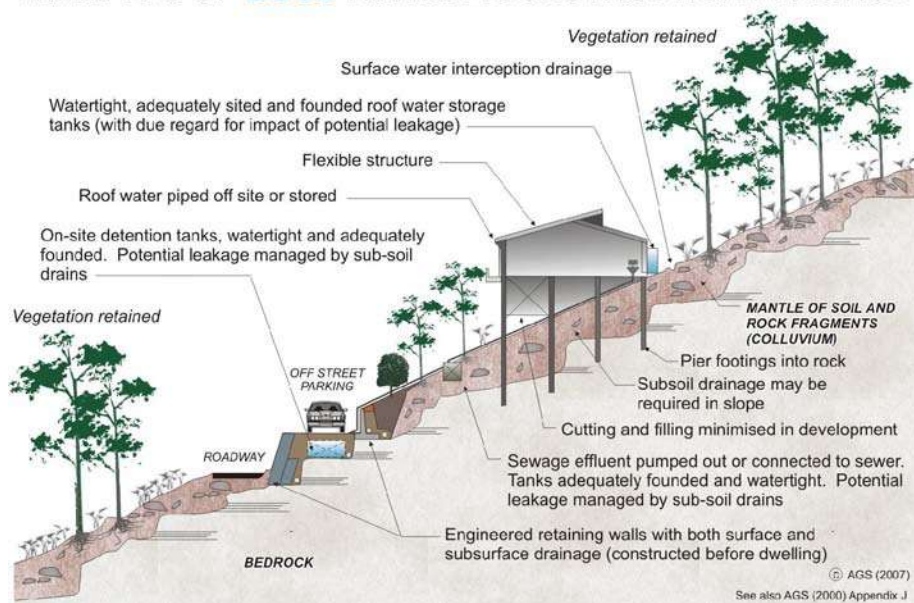
Journal and News of the Australian Geomechanics Society
Volume 42 No 1 March 2007

Extract containing:

“Practice Note Guidelines for Landslide Risk Management 2007”

Ref: AGS (2007c)

EXAMPLES OF **GOOD** HILLSIDE CONSTRUCTION PRACTICE



Landslide Risk Management



PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

**Australian Geomechanics Society Landslide Taskforce,
Landslide Practice Note Working Group**

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PART A: BACKGROUND

1 INTRODUCTION

1.1 PREAMBLE

Slope instability occurs in many parts of urban and rural Australia and often impacts on housing, roads, railways and other development. This has been recognised by many local government authorities, and others, and has led to the requirement by many local government councils for stability assessments prior to allowing building development.

In 2000, the Australian Geomechanics Society (AGS) published “Landslide Risk Management Concepts and Guidelines” (AGS 2000). Since then there have been many published papers and discussion which have progressed Landslide Risk Management (LRM) in particular and risk management in general. As a consequence, AGS considered it appropriate to develop more comprehensive guidelines for practitioners and regulators involved in LRM.

This Practice Note Guidelines for Landslide Risk Management (the Practice Note) and its Commentary (AGS 2007d) are one part of a series of three guidelines related to LRM that have been prepared by AGS with funding under the National Disaster Mitigation Programme (NDMP). That programme has been introduced by the Australian Government to fund disaster mitigation, addressing hazards such as flooding, bushfires and landslides.

The associated guidelines which should be read in conjunction with the Practice Note are:-

- AGS (2007a) “*Guideline for Landslide Susceptibility, Hazard and Risk Zoning for Land Use Planning*”.
- AGS (2007e) “*Australian GeoGuides for Slope Management and Maintenance*”.

1.2 PURPOSE

The purpose of this Practice Note is to:

1. Review the Australian Geomechanics Society (AGS) Landslide Risk Management Concepts and Guidelines (AGS 2000) in the light of usage since publication and update accordingly and in addition, to take the opportunity to establish a formal revision process/documentation. Accordingly, a Revision Table is included in the Practice Note.
2. Provide guidance and recommendations on tolerable risk criteria, minimum reporting standards and assessment criteria/options to Local Government and Government bodies who as the regulator, receive Landslide Risk Management (LRM) reports and decide on levels of Tolerable Risk.
3. Provide guidance of a technical nature in relation to the processes and tasks undertaken by geotechnical practitioners who prepare LRM reports including appropriate methods and techniques. The Practice Note is a statement of what constitutes good practice by a competent practitioner for LRM, including defensible and up to date methodologies.
4. Provide guidance on the quality of assessment and reporting, including the outcomes to be achieved and how they are to be achieved. It sets out the functions and responsibilities of the professional carrying out the assessment.
5. Be a reference document for legislative purposes, which has been subject to nation-wide peer review.

1.3 SCOPE

This Practice Note supersedes AGS (2000) as the guideline for good practice and is accompanied by a Commentary (AGS 2007d) which discusses various aspects and gives appropriate references, and which should be read in conjunction with this Practice Note.

AGS (2000) contains much useful and relevant commentary which can (and should) be read in conjunction with the Practice Note. It is not the intention of the Practice Note to supersede this valuable commentary, rather to complement it. AGS (2000) should be regarded as “companion literature”. Unless specifically discussed or revised in the Practice Note, the Working Group considers the commentary, examples and references provided in AGS (2000) to constitute appropriate background for the use of the Practice Note.

The emphasis of the Practice Note is on residential subdivision and development, particularly when considering the requirements for assessment on a lot-by-lot basis for either existing or proposed development.

The recommendations are however applicable to all classes of urban and rural building development or the environment.

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

The risk analysis principles could be adopted for short term risks associated with trenches or excavations during construction projects and for quarries and open cut mines. For such cases, risk tolerance criteria are controlled by occupational health and safety requirements and are not covered here.

The Practice Note can be applied to roads and railways. However, special consideration has to be given to the number of users, their temporal spatial probability and the summation of the risk along the route. This is discussed further in the Commentary.

1.4 CONVENTIONS USED

The Practice Note includes imperative verbs, such as ‘establish’, ‘use’, ‘identify’ and so on. These are to be understood as meaning; “*AGS recommends that you establish...*”, or “*...that you use...*” or “*...that you identify....*” and so on as the case may be. This form of expression has been used to avoid unnecessary repetition of wording in the sense of ‘plain English’.

Paragraphs presented in **bold type** constitute the guideline statement and subsequent sub paragraphs provide discussion of the guideline topic. Further discussion is provided in the Commentary.

In the following, use of the word ‘landslide’ implies both existing (or known landslides) and potential landslides which a practitioner might reasonably predict based on the relevant geology, geometry and slope forming processes. Such potential landslides may be of varying likelihood of occurrence. ‘Landslide’ also includes ‘landslip’ (as used in Victorian legislation), ‘slump’ and the various landslide forms (see Appendix B).

1.5 STAKEHOLDERS

The various stakeholders who may be affected by landslide risk include:-

- The **landowner** who will frequently be the client in terms of a commission to prepare a LRM report for a site or a development proposal.
- The **occupier** who would most often also be the land owner.
- The **financier** who would often be a financial institution having an interest in the land and any development thereon.
- The **regulator** (Appendix A) who would have responsibility for setting risk acceptance criteria, administering planning controls and approving development proposals as being within the requirements of planning controls, or a policy.
- The **practitioner** (Appendix A) who would have the required expertise for and responsibility of preparing a LRM report and recommending suitable risk control measures, when needed, to achieve the risk acceptance criteria.
- The **design professional** (such as architect or structural engineer) who would be one of the advisors to the client with responsibility for integration of risk control measures recommended by the practitioner into the development scheme, where possible, within the design brief from the client.
- The **insurer** where appropriate may have an interest in providing insurance cover against nominated insurable risks.

Although there is no section in the Practice Note dealing with the Client, clearly the Client is an essential stakeholder in relation to the practitioner. The Client will be relying on unbiased, sound technical advice from the practitioner as to the risk that a development proposal poses to the client and /or his interests. It will be the responsibility of the client to accept the risks involved, subject to the approvals of the regulator.

2 RISK TERMINOLOGY

The framework for the LRM process, as shown in Figure 1 in a simplified flow chart form, should be adopted.

Adopt the recommended terminology for ease of communication and clarity as defined in Appendix A.

As with most areas of expertise, there is a technical jargon associated with LRM. Specialist terminology is used to convey succinct ideas or facts. This cannot be avoided and by necessity is of a technical nature. The relevant terminology is defined in Appendix A. The lay reader is also referred to the Commentary for further discussion and to the GeoGuides (AGS 2007e).

This Practice Note, and the companion AGS guidelines (AGS 2007a, 2007e), use the term ‘landslide’ rather than ‘landslip’ or ‘slump’ or similar, to cover a wide range of failure mechanisms in soil, rock (as discussed in Appendix B) and man made structures such as retaining walls, as implied by the definition in Appendix A.

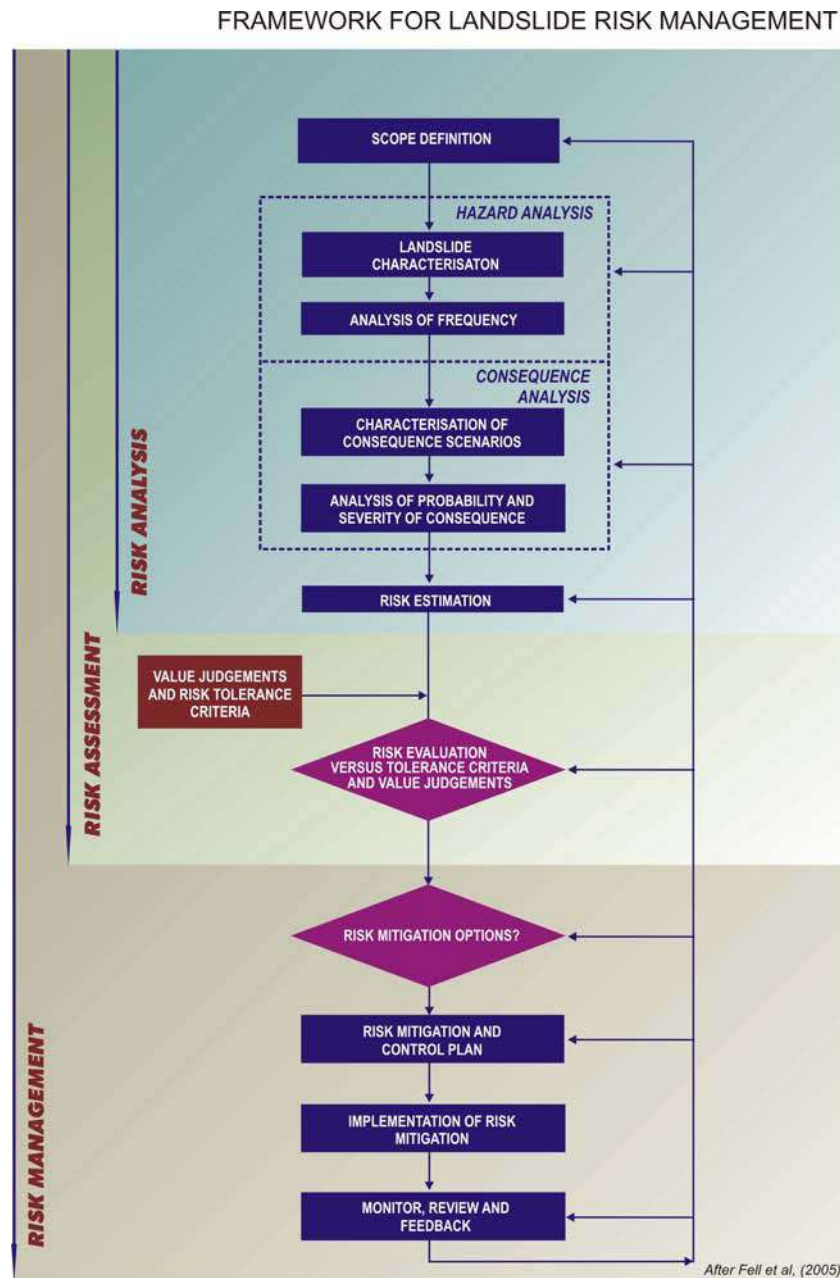


Figure 1.

The Framework for LRM presented in Figure 1 is similar to the flow chart in AGS (2000). However, it has been simplified in presentation and has been amended slightly from AGS (2000) to reflect the inclusion of Frequency Analysis as part of Hazard Analysis (in accordance with the abovementioned definition of hazard and as defined in AGS 2000).

Definitions for associated terminology have also been included in Appendix A together with an explanation of Landslide Risk as presented in AGS Australian GeoGuide LR7.

PART B GUIDELINES FOR REGULATORS

3 GUIDELINES FOR REGULATORS

3.1 BACKGROUND

The term landslide denotes “the movement of a mass of rock, debris or earth down a slope”. The phenomena described as landslides are not limited to either “land” or to “sliding” and usage of the word has implied a much more extensive meaning than its component parts suggest. The rates of movement cover the full range from very rapid to extremely

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slow. The size, similarly, can vary enormously. The combination of type of landslide, size and rate of movement can determine the destructive power, and hence potential consequences of the landslide in terms of damage to property, loss of life, economic costs and impact on the environment. Subsidence, as a mechanism, is excluded from consideration, though it may be similar in consequence and appear to be of a similar form. Appendix B presents a summary of the terminology used to classify and describe landslides.

Landslides can impact on human development and activity as well as natural areas / features. It is the potential impact on human development which becomes of concern to the planners, regulators and disaster management authorities. Landslides can be just one of a number of threats which have to be considered, others being for example flooding, bush fires, and seismicity.

Examples of where landsliding is potentially an issue include:-

- a) Where there is a history of landsliding.
- b) Where there is no history of sliding but the topography dictates sliding may occur.
- c) When there is no history of landslides but geological and geo-morphological conditions are such that sliding is possible.
- d) Where there are constructed features which, if they fail, may travel rapidly.
- e) Forestry works and agricultural land clearing which can lead to landslides causing damage to the environment.

Specific examples of the above are given in the AGS Guidelines for Landslide Susceptibility, Hazard and Risk Zoning for Land Use Planning (AGS 2007a). AGS (2007a) also provides detailed guidance to the regulator in relation to landslide zoning for planning purposes.

3.2 RELEVANCE TO APPROVALS PROCESS

Details of the approvals process may vary in detail from state to state. It is understood that in all States and Territories of Australia, the regulator has a statutory responsibility to consider the impact of a number of hazards, including landslides, on potential development of land as a 'duty of care' exercise. The regulator is usually the local government, but may be a State Government department or body. The actual mechanism and regulatory context for dealing with planning controls, building controls and approval process varies from state to state. However, the outcome should be that areas having a landslide risk are properly considered in relation to land use and development proposals.

In order to develop planning controls and building regulations, local government (or other regulators) must ensure that it has the statutory means to:

- a) Through a planning scheme and using the principles in AGS (2007a), identify the areas that are susceptible to or at risk from landslides.
- b) Require planning and/or building approvals for all land use and development within the areas zoned as susceptible to landslides.
- c) Ensure there is a proper process for assessment in relation to existing and proposed development, including the requirement for completion of LRM reports in accordance with this Practice Note.
- d) Provide appropriate risk tolerance criteria for loss of life and property so that there is a means to determine whether it is appropriate for development to occur or the required land use to proceed.
- e) Apply, if necessary, consent conditions on the land use and/or development approval, including conditions requiring maintenance that will appropriately manage the landslide risk for that use and/or development.

It can be seen from the above that zoning in accordance with AGS (2007a) becomes the 'initiator' under the planning scheme and building approvals process to determine whether LRM controls are required and whether more detailed LRM consideration is required.

3.3 POLICY REQUIREMENTS

The regulator should have a specific policy which sets out the requirements for LRM assessments as part of the development application documentation and process.

The need for such a policy should be determined by zoning studies in accordance with AGS (2007a). Essential components of such a policy will include:

- 3.3.1 When a LRM assessment is required.** This may be related to a Susceptibility or Hazard Zoning Study or some other plan or criteria defining areas or types of development included or excluded.
- 3.3.2 The necessary competencies of practitioners undertaking LRM assessments. Such practitioners should be required to have LRM as a core competency.** A method of demonstrating core competency in LRM is being addressed by the Australian Geomechanics Society and Engineers Australia as a specific area of practice within the National Professional Engineers Register (NPER). Some regulators may choose to define another method of demonstrating competency.
- 3.3.3 The basic requirements of LRM reports** which should be based on compliance with the requirements of this Practice Note.

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- 3.3.4 **Require assessment of risk to life as part of a LRM report** which, as discussed below, should be completed in a quantitative basis.
- 3.3.5 **Suggest adoption of the preferred qualitative terminology given in Appendix C of this Practice Note for risk to property** so that the regulator can become accustomed to the terminology adopted and implications arising there from. If alternative terminology is to be adopted for LRM, the regulator should only accept non standard schemes where the terms have been clearly defined, the terms have been explained in relation to the preferred terminology and it can be reasonably demonstrated by the practitioner that the alternative is better suited to the particular circumstances of the assessment.
- 3.3.6 **Provide the required forms** to control the submissions and approvals process.
- 3.3.7 **Specify the criteria under which a decision will be made for both the scope/nature of developments and the appropriate tolerable risk criteria being adopted.**

3.4 PROCESSING REQUIREMENTS

- 3.4.1 **The regulator should use a number of forms to provide appropriate QA process control and documentation records of the submitted LRM assessment and subsequent compliance with the approval conditions.**

The forms need to be appropriate to each stage of the development application, approval, detailed design, construction and maintenance of the development. Essential contents will include:

1. Name and qualification of the practitioner responsible for the LRM assessment.
2. A list of supporting documents including the architectural, civil design and structural engineering design drawings, as appropriate, to fully define the extent and scope of the proposed development.
3. A statement of compliance with the requirements of this Practice Note. In some cases the statements will be required to include details of how compliance is achieved.
4. Document reference details (date, reference number, report title) for the relevant LRM assessment submission.

A suite of example forms is given in Appendix D for modification by each regulator to be consistent with their policy. The aim of the forms is to provide appropriate documentary control of the stages required through to completion of a development.

Processing of the application by the regulator should include, amongst other aspects, confirmation that the submission is in accordance with policy requirements, and that the nature of the development complies with the requirements of the LRM assessment.

Where the regulator has specific concerns in relation to the adequacy of a submission, or the conclusions reached, or if required by a Hazard Zoning study, the submission may be subject to peer review or independent specialist advice to the regulator as an audit process or as part of mediation for an agreement. The reviewer should independently review the LRM assessment report in terms of adequacy of compliance with this Practice Note and the reasonableness of the assessment conclusions and risk control measures specified. The review should also consider the specific development proposals as defined by the design drawings.

- 3.4.2 **Where the recommendations of this Practice Note have not been followed, then the regulator should either reject the application or require provision of further information before approval is given.**

It is anticipated that the forms in Appendix D will, in part, constitute a checking template for the regulator. Further discussion is given in the Commentary.

- 3.4.3 **Where construction is completed but all aspects of the Approval Conditions have not been completed with appropriate documentation or justification, then the final approval by the regulator should not be given until sufficient information is provided to demonstrate compliance.**

It is anticipated that completion of Forms F and G with suitable annotation would help identify where non compliance exists. If the regulator does not have a strong procedure for enforcement of, or auditing of, compliance with consent conditions, then there may be subsequent liability issues for the regulator if non-compliance becomes an issue at a later date.

3.5 ESTABLISHMENT OF TOLERABLE RISK CRITERIA

The regulator is responsible for setting the Tolerable Risk Criteria for loss of life and property loss. Discussion of the considerations and world practice are given in the Commentary together with the AGS recommendation for consideration by the regulator.

3.6 LANDSLIDE INVENTORY

The local Council, or other regulator, should maintain an inventory of past landslide events as discussed in AGS (2007a) and make this information available to all practitioners.

3.7 ROLE AND RESPONSIBILITY OF THE PRACTITIONER

The practitioner has the role of providing technical input in relation to the specialized aspect of LRM. Such input will be subject to the specific requirements of any policy instituted by the regulator. The regulator may require specific levels of qualification and competence of practitioners providing the regulator with advice in relation to compliance with the risk acceptance criteria.

The qualifications and experience of suitable practitioners are as discussed in Paragraph 3.3.2.

It is the responsibility of the practitioner to carry out LRM assessments in accordance with this Practice Note and within the requirements of his/her professional Code of Ethics. The practitioner must provide advice to the client and regulator in an unbiased manner.

PART C GUIDELINES FOR PRACTITIONERS

4 SCOPE DEFINITION

Establish the purpose and scope of the risk assessment study.

The practitioner needs to take into account the initial brief from the client and the requirements of the regulator. Usually these will be sufficient for the practitioner to decide on the appropriate scope and level of the study which should then be advised to the client as a “reverse brief”. In the LRM process, the practitioner will have a role to advise the client as to how the landslide risk can be reduced, avoided or otherwise controlled including options or alternatives.

5 HAZARD ANALYSIS

5.1 DATA GATHERING / DESK STUDY

Assemble relevant data and record their sources.

Often there is a body of local experience which becomes invaluable for the assessment process. Such experience includes published papers, geological maps, aerial photographs and general studies such as Hazard Zoning studies completed for the regulator. Local experience can include previous assessments and knowledge of problematic areas which should be available from the regulator’s landslide inventory. Practitioners new to an area should discuss with locals their knowledge and experience.

Preferred data for the assessment will include site specific data, such as survey plan showing existing features, spot heights, contours and location and nature of services. Initial design proposals are required so that the risk assessment may be completed and appropriate risk control measures specified. (It is a necessary requirement in the performance of a risk assessment for there to be an element at risk, hence the need for a preliminary design or for an assumed development which should be defined in the LRM report).

5.2 FIELD INVESTIGATION REQUIREMENTS

5.2.1 Complete investigations sufficient to establish a geotechnical model, identify geomorphic processes and associated process rates.

The investigation may involve a number of methods and may be completed in stages, with each stage sufficiently detailed to provide a model appropriate to the level of study being undertaken. Further discussion is given in the Commentary.

5.2.2 Inspect the site and surrounds including field mapping of the geomorphic features.

This must be completed by the practitioner for every assessment. The field mapping is to document the observations and to enable formulation of the geotechnical model.

Mapping should be completed to scale on an available survey plan and must include the surrounds (above, below and adjacent) to the site as appropriate to define the landslides and the geotechnical model.

Where a survey plan is not available, then simple survey using hand held tape and clinometer methods should be used to draw up a plan, to scale, using standard mapping symbols and terminology to represent the geological and geomorphic features. (Examples of geological and geomorphic mapping symbols are presented in Appendix E.)

5.2.3 Determine the subsurface profile from exposures or subsurface investigation such as by boreholes and/or test pits.

This is necessary as part of the geotechnical model. Often exposures or knowledge from a nearby site may be sufficient.

Where such data is not available or not appropriate, subsurface investigation is required to enable formulation of the model and must include determination of the depth to rock or to below the depth of potential failure surfaces if this is greater.

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Where pre-existing landslides are expected or suspected, then where practical, use should be made of either test pits (to enable sufficient sample/material to be seen for identification of shear planes or other relevant structure) or boreholes (with appropriate sampling and installation of inclinometers for monitoring for evidence of movements).

5.2.4 Assess likely groundwater levels and responses to trigger rainfall events.

Consideration of the likely ground water response will enable assessment of response to rainfall trigger events. Use may be made of experience in the area, as observation of site specific data will frequently require prolonged periods of monitoring to enable formulation of a groundwater response model taking into account the statistical significance of rainfall events during the monitoring period. For relatively straightforward projects with low to moderate risks, a basic qualitative estimate of groundwater levels and responses may be appropriate when there is a lack of data. However, other more complicated projects, or where risk levels are higher, will require a greater level of understanding of groundwater levels and responses.

For more detailed analysis, particularly of possible stabilisation measures by subsurface drainage, observation of groundwater levels and their response to significant rainfall events is advisable to enable subsequent assessment of the effectiveness of subsurface drainage measures. Careful consideration must be given to the location of piezometers and their construction details.

5.2.5 Prepare a cross section drawing (to scale) through selected parts of the site to demonstrate the geotechnical model of site conditions and on which landslides may be identified.

The resulting geotechnical model should integrate all the data obtained from the mapping and investigations.

The section should demonstrate the likely variation in subsurface conditions on the section including groundwater levels. On large or complex sites, more than one section may be required. All sections are to be drawn to natural scale. If exaggerated vertical scale is required for clarity, then a summary section at natural scale should also be included.

Adequate investigation has been completed when the geotechnical model is sufficiently defined to understand the slope forming processes relevant to the site and surrounds, the form and extent of landslides, likely triggers for the landslides and process rates associated with the landslides. The report should include explanation of uncertainties associated with the model.

5.2.6 Take into account slope forming process rates associated with the geotechnical model and landslides.

An understanding of the slope forming process relevant to the landslides and associated process rate is fundamental for evaluation of likelihood.

5.2.7 Identify landslides types/locations appropriate to the geotechnical model based on local experience and general experience in similar circumstances.

The types of landslides will be dependent on the geotechnical model and to some extent on the nature of existing and/or proposed development. The expected characteristics of the landslides (such as the size, type of material involved, rate of failure and travel distance) need to be assessed. The range of landslide sizes can vary from the very large landslides, which may encompass a whole hillside or region, to a small site specific landslide. The model should include assessment of the fundamental cause as well as likely trigger events. The report must document the hazard assessment which will include the estimated likelihood for each landslide type.

The hazard assessment must address areas upslope from the site, downslope from the site and across the slope adjacent to the site where these may affect the site.

5.2.8 If required, further detailed investigations should be completed to better define the model, the landslides, the triggers, the frequency (likelihood) or design of stabilisation measures to control the risk.

Such additional investigation is most likely to be required on sites where the risk is judged to be intolerable and/or where further input is required to resolve uncertainties.

5.3 LANDSLIDE CHARACTERISATION

Characterise the landslides based on the desk study and field investigations. Use Appendix B for terminology to describe the landslides.

The characterization should include the classification, volume, location and potential travel distance of all landslides which may occur on the site or travel on to or regress into the site.

5.4 FREQUENCY ANALYSIS

5.4.1 Techniques for Frequency Analysis

a) Adopt a frequency analysis technique appropriate to the level of study and complexity of the geotechnical model and slope forming process.

The appropriate technique may change with different levels of study, or for different stages of a project, or with the project brief and available budget. For example, techniques and level of detail may be different for:

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- Subdivision stage LRM
- Residential dwellings LRM
- Infrastructure and utilities LRM
- Natural resource and environmental LRM

It is essential that the assessment be based on the best estimates available and that expert judgment be applied to answers so derived.

It is essential to understand the slope forming process before moving on to the frequency assessment.

The assessment must document the reasoning in a transparent manner.

b) Gather local and historical knowledge of slope performance and landslide characteristics and occurrence. The resulting inventory enables assessment of frequency.

This technique is a basic starting point and essential for all studies. However, a common shortcoming is that “local knowledge” is often poorly documented and difficult to collate and assess. Local Council records and experience should be accessed via a landslide inventory made available to practitioners. Analysis of aerial photographs and possibly maps may provide additional data.

Documentation of events by local newspapers may also be a useful source, depending on the quality of reporting and what events are judged at the time to be of local interest.

c) Empirical methods based on slope instability ranking systems.

These methods are often devised by expert groups to assist with prioritisation of treatment measures.

The methods are usually based on subjective judgment of the relative importance of contributory factors. The results obtained may be difficult to calibrate or it may be difficult to obtain consistent results and hence may be inaccurate. The methods do not usually allow assessment of frequencies.

d) Relationship to geomorphology and geology.

This method is based on the principle put forward by Varnes (1984) that the past and present are guides to the future. Hence, this leads to the assumptions that:

1. it is likely that landsliding will occur where it has occurred in the past and
2. landslides are likely to occur in similar geological, geomorphologic and hydrological conditions as they have in the past.

The use of historic records and landslide inventories of past performance are likely to be required to enable frequency values to be assessed. However, it should be noted that landslide frequency, size and intensity may differ from past performance where altered trigger events are introduced, e.g. due to man made changes or climate change. In addition, other factors (such as periodic or seasonal wetting and drying cycles resulting in soil creep, cyclic degradation and strength loss) can also result in failures after relatively “normal” rainfall events.

The use of other slope attribute factors (such as slope angle, slope drainage, slope age, presence of groundwater, slope orientation) may assist with assessment of particular slopes relative to the broad geomorphic model.

e) Prepare a statistical evaluation of rainfall and relate to history of landsliding and population of slopes within area of similar slope type.

Rainfall, and the consequent effect on groundwater levels, is widely recognized as a main trigger event for landsliding. Therefore, indicative frequency values may be related to the frequency of rainfall provided there is sufficient historical data to enable the relationship between rainfall frequency, antecedent rainfall and landslide events to be correlated.

A similar approach may be adopted for other forms of triggering events such as earthquakes.

f) Consider use of simulation models and Monte Carlo sampling analyses to derive a frequency of failure.

These methods (including simulation modelling of groundwater response to rainfall, evapotranspiration, and ground water flows) can be difficult to carry out reliably. Picarelli *et al.* (2005) outline some of the difficulties with these methods. Simulation modelling is most likely to be applicable only to medium to large, deep seated landslides where extensive monitoring data is available to enable calibration over a range of rainfall and piezometric responses.

Experience shows that full probabilistic analysis is difficult and time consuming (Robin Fell personal comm.). Therefore this method should only be carried out for special cases where sufficient data is available to enable the results to be meaningful.

g) Use knowledge based expert judgment or ‘degree of belief’ method which combines experience, expertise and general principles.

For most assessments this may be the only suitable option to estimate frequency due to the lack of objective data. The assessment relies to a large degree on subjective assessment of available data where other more rigorous methods are not available or viable. The method still requires some degree of research to obtain relevant data and an understanding

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of the geological model to qualify the judgment of likelihood. Nonetheless, the approach requires the proposition of various possible scenarios followed by the systematic testing and elimination of options as a result of investigation, discussion and judgment to develop an estimate of frequency (Lee and Jones 2004).

The result is conditioned by the 'degree of belief' of the practitioner. Typically, the resulting accuracy for a frequency assessment and, perhaps, a consequence assessment could vary from half an order of magnitude at best, to one order of magnitude or perhaps two orders of magnitude. As a result, the risk assessment should clearly display its sensitivity to the input parameters and, unless justified by further investigations, a conservative outcome should be adopted.

- h) Where appropriate, use event trees to provide a structured and auditable approach for the use of expert judgment and subjective probability assessment.**
- i)**

An event tree analysis uses a graphical construct to show the logical sequence of events or considerations that can be used to analyse the system leading to a particular outcome. It can be used for evaluation of probability of failure of a landslide, or consequence of failure, or risk. The logical sequence within the system is mapped as a branching network with conditional probabilities assigned to each branch of a node. The frequency of achieving a certain outcome is the product of the conditional probabilities leading to that outcome times the frequency of the initiating "trigger" such as rainfall.

i) Other methods.

The above may not be an exhaustive list but covers the principal methods/approaches. Specific circumstances of a particular area or project may enable other approaches or combinations of approaches to be used. Field techniques may develop to offer alternatives, for example remote sensing by satellite.

Further comment is given in the Commentary together with some guidance on different site investigation methods.

5.4.2 Estimation of Annual Probability (Frequency) ($P_{(H)}$) of Each Landslide

a) Use 'best estimates' for frequency but consider range / uncertainty / sensitivity.

Suitable methods are outlined in Section 5.2.

It is important not to infer greater accuracy than is reasonably possible. Evaluation of the sensitivity arising from uncertainty is part of the consideration.

A best estimate is to be derived for each landslide which is then applied to both risk to property and risk to life assessments. The estimate may be related to the size of the landslide and/or the expected amount of movement as part of the hazard assessment. The appropriate qualitative term is chosen from the estimated probability based on the frequency assessment. Note that the reverse, the adoption of a probability value from a qualitative term, should not be undertaken as it has been demonstrated that this results in a range of estimates of frequency several orders of magnitude apart depending on the practitioner.

b) Estimates of frequency may be derived by partitioning the problem to (Annual probability of trigger event) x (Probability of sliding given the trigger event) over the range of trigger events.

Landslides of the one 'type', but having varying possible scales (magnitude/travel distance/velocity etc.) need to be assessed separately. Each could well have a different frequency of occurrence. The landslide inventory of performance for an area will provide some basis for the assessment.

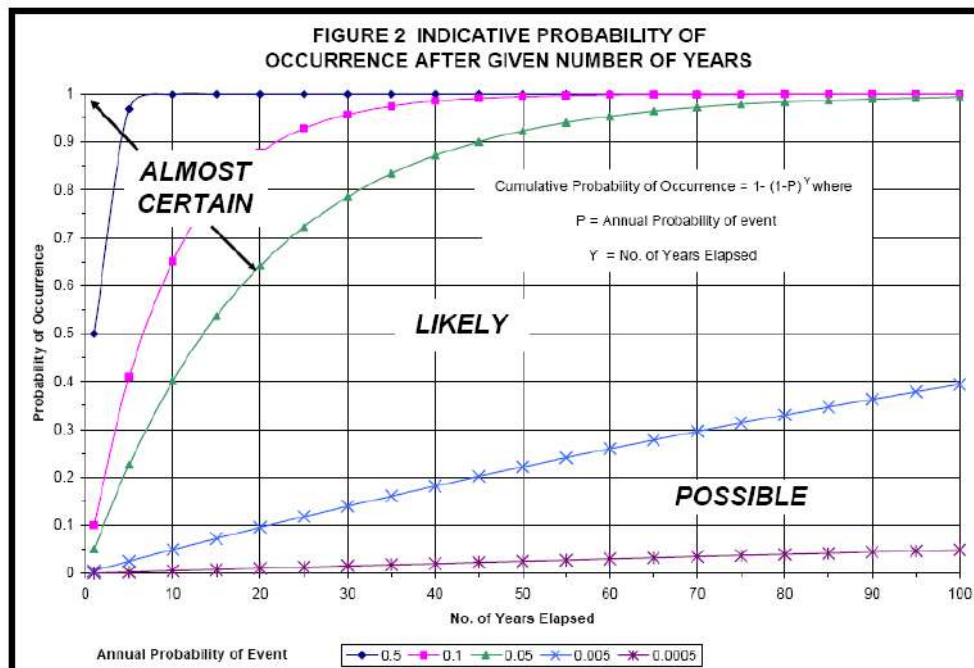
A trigger event for a particular locality (e.g. a certain intensity/duration or recurrence interval of rainfall) will not necessarily cause each potential landslide event in that locality to occur. There will be a finite probability (value) that the landslide under consideration may not be set off by the trigger event.

The frequency of landsliding should be assessed over the full range of the triggering events, and the total frequency carried forward in the risk analysis. In practice this process may be simplified to consider only the highest frequency triggering events. An example is presented in the Commentary.

c) Complete a review of the assessed frequency in relation to the implied cumulative frequency of the event occurring within the design life and known performance within the area.

This is a 'sanity check' on the result of the assessment. It is important to apply judgment or bias on the final outcome only, not on the input estimates.

Values of the cumulative probability are shown on Figure 2 for different annual probability values as a function of time over usual design life intervals. The resulting cumulative probabilities should be checked to confirm they are reasonable in relation to experience. The implications of the cumulative probability values shown in Figure 2 are discussed further in the Commentary.



5.4.3 Assess the Travel Distance and the Probability of Spatial Impact ($P_{(S;H)}$) of the Elements at Risk

When assessing risk arising from landsliding, it is important to be able to estimate the distance the slide mass will travel and its velocity. These factors determine the extent to which the landslide will affect property and persons downslope and the ability of persons to take evasive action.

The travel distance depends on:

- Slope characteristics
 - Height
 - Slope
 - Nature of material
- Mechanism of failure and type of movement such as
 - Slide, fall, topple etc.
 - Sliding, rolling, bouncing, flow
 - Strain weakening or not
 - Collapse in undrained loading (static liquefaction)
 - Influence of surface water and groundwater
 - Comminution of particles
- Characteristics of the downhill path
 - Gradient and gradient direction
 - Channelisation
 - The potential for depletion/accumulation
 - Vegetation

Information on travel distance from previous events on or near the site may be collected during the site inspection. Predictions of travel distance and travel direction should be based on the assessed mechanism of future events and site characteristics.

For rotational landslides which remain essentially intact, the method proposed by Khalili *et al* (1996) or experience with landslides in similar geological, topographic and climatic conditions can be used to estimate the displacement. Further discussion is given in the Commentary.

For slides which break up, and in some cases become flows, and slides from steep cuts, the travel distance is usually estimated from empirical methods, such as Hunter and Fell (2002) and Corominas (1996). These methods are only approximate, and the wide scatter of data on travel distance angles reflects the range of topographical, geological and climatic environments, different slide mechanisms and limited quality of data from which the methods are derived.

If the empirical methods are to be used for predictions of travel distance and the probability of spatial impact of the elements at risk, much judgement will be required and it is important to try to calibrate the methods with landslide

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behaviour in the study area. It is often useful to allow for a range of travel distances in the calculation and express that range in probabilistic terms as discussed in the Commentary.

The annual probability of the landslide and probability of spatial impact may be considered together in qualitative terms as likelihood of impact on the element at risk being considered.

6 CONSEQUENCE ANALYSIS

6.1 ELEMENTS AT RISK

The elements at risk will include:

- Property, which may be subdivided into portions relative to the hazard being considered.
- People, who either live, work, or may spend some time in the area affected by landsliding.
- Services, such as water supply or drainage or electricity supply.
- Roads and communication facilities.
- Vehicles on roads, subdivided into categories (cars, trucks, buses).

These should be assessed and listed for each landslide hazard.

For some cases, other risks may also have to be considered. For example:

- Environmental, where the elements at risk are environmental (rather than man made), such as forests or water bodies.
- Social, where the consequences of the landslide may have an impact on social conditions, such as the cost of disruption to traffic where roads are affected.
- Political, where the consequences may not be acceptable in political terms.

6.2 TEMPORAL SPATIAL PROBABILITY ($P_{(T:S)}$)

When the elements at risk are mobile (e.g. persons on foot, in cars, buses and trains) or where there is varying occupancy of buildings (e.g. between night and day, week days and weekends, summer and winter), it is necessary to make allowance for the probability that persons (or a particular number of persons) will be in the area affected by the landslide. This is called the Temporal Spatial Probability.

For where the elements at risk are mobile it is proportion of a year (between 0 and 1.0) in which a person, car or bus will be below or on the landslide when it occurs. For occupancy of buildings it is a calculation of the proportion of a year (between 0 and 1.0) which the number of persons being considered occupy the building, or the area of the building likely to be impacted.

These calculations should allow for the possibility that the persons may have warning of the impending landslide and may evacuate the area. Each case should be considered by taking account of the details of the situation. Generally persons on a landslide are more likely to observe the initiation of movement and move off the slide, than those who are below a slide which falls or flows onto them unless the rates of movement are slow.

6.3 EVALUATION OF CONSEQUENCE TO PROPERTY

6.3.1 Estimate the extent of damage likely to property arising from each of the landslides.

This requires an understanding of the landslide characteristics and experience in assessing the likely impact on property. The consequences are often calculated using the vulnerability ($V_{(Prop:S)}$) of the elements at risk to the landslide.

The factors which most affect vulnerability of property are:

- The volume of the slide in relation to the element at risk.
- The position of the element at risk, e.g. on the slide, or immediately downslope.
- The magnitude of slide displacement, and relative displacements within the slide (for elements sited on the slide).
- The rate of slide movement.

It should be noted that the vulnerability refers to the degree of damage (or damage value in absolute or relative terms) which is judged to be likely if the landslide does occur.

As discussed below, the assessment should be based on a quantitative estimate to enable clarification of the judgment which for a qualitative assessment may be subject to considerable interpretation.

6.3.2 Estimate the indicative cost of the damage.

This requires use of indicative costs of building and remedial works. Frequently, broad brush 'guesstimates' will suffice, but the 'guesstimate values' and basis should be documented. Some guidance is given in the Commentary. It should not be necessary to use a quantity surveyor to establish a more accurate estimate as usually the broad brush guesstimate will suffice for allocation of a consequence term in a qualitative scheme such as in Appendix C.

The indicative cost of damage is to be the Total Cost as this is the most relevant to the owner. Components to be considered comprise:-

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- Direct costs related to reinstatement works for damaged portions of the property (structures and the land).
- Stabilization works required to render the site to an tolerable risk level for the landslide.
- Professional and approvals fees.
- Consequential costs (such as legal fees and alternative temporary accommodation).

It does not include additional stabilisation works to address other landslides which may affect the property.

6.3.3 Estimate the market value.

This may be achieved by reference to property sale values within the local area which will reflect the value of the land plus structures. The client is likely to have some knowledge of the local market values. Again, a broad-brush guesstimate should often suffice.

6.3.4 Consider the resulting Consequence classification, such as using Appendix C, and implied accuracy of the above estimates.

It is not expected that the assessor will be a quantity surveyor or have similar experience, but that sensible estimates, possibly as a range, can be made and documented. Statement of limits of accuracy or uncertainty are appropriate for sensitivity and appraisal analysis.

6.4 EVALUATION OF CONSEQUENCES TO PERSONS

The following factors influence the likelihood of deaths and injuries or vulnerability ($V_{(D:T)}$) of persons who are impacted by a landslide:

- Volume of slide.
- Type of slide, mechanism of slide initiation and velocity of sliding.
- Depth of slide.
- Whether the landslide debris buries the person(s).
- Whether the person(s) are in the open or enclosed in a vehicle or building.
- Whether the vehicle or building collapses when impacted by debris.
- The type of collapse if the vehicle or building collapses.

Persons are very vulnerable in the event of complete or substantial burial by debris, or the collapse of a building. It should be noted that even small slides, and single boulders, can kill people.

Appendix F provides some indicative examples of vulnerability values. The Commentary provides some more detailed discussion.

7 RISK ESTIMATION

7.1 QUANTITATIVE RISK ESTIMATION

Quantitative risk estimation involves integration of the frequency analysis and the consequences.

For property, the risk can be calculated from:

$$R_{(Prop)} = P_{(H)} \times P_{(S:H)} \times P_{(T:S)} \times V_{(Prop:S)} \times E \quad (1)$$

Where

$R_{(Prop)}$ is the risk (annual loss of property value).

$P_{(H)}$ is the annual probability of the landslide.

$P_{(S:H)}$ is the probability of spatial impact by the landslide on the property, taking into account the travel distance and travel direction.

$P_{(T:S)}$ is the temporal spatial probability. For houses and other buildings $P_{(T:S)} = 1.0$. For Vehicles and other moving elements at risk $1.0 > P_{(T:S)} > 0$.

$V_{(Prop:S)}$ is the vulnerability of the property to the spatial impact (proportion of property value lost).

E is the element at risk (e.g. the value or net present value of the property).

For loss of life, the individual risk can be calculated from:

$$R_{(LoL)} = P_{(H)} \times P_{(S:H)} \times P_{(T:S)} \times V_{(D:T)} \quad (2)$$

Where

$R_{(LoL)}$ is the risk (annual probability of loss of life (death) of an individual).

$P_{(H)}$ is the annual probability of the landslide.

$P_{(S:H)}$ is the probability of spatial impact of the landslide impacting a building (location) taking into account the travel distance and travel direction given the event.

$P_{(T:S)}$ is the temporal spatial probability (e.g. of the building or location being occupied by the individual) given the spatial impact and allowing for the possibility of evacuation given there is warning of the landslide occurrence.

$V_{(D:T)}$ is the vulnerability of the individual (probability of loss of life of the individual given the impact).

A full risk analysis involves consideration of all landslide hazards for the site (e.g. large, deep seated landsliding, smaller slides, boulder falls, debris flows) and all the elements at risk.

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For comparison with tolerable risk criteria, the individual risk from all the landslide hazards affecting the person most at risk, or the property, should be summed.

The assessment must clearly state whether it pertains to 'as existing' conditions or following implementation of recommended risk mitigation measures, thereby giving the 'residual risk'.

7.2 SEMI-QUANTITATIVE AND QUALITATIVE RISK ESTIMATION FOR RISK TO PROPERTY

When considering the risk to property, it may be useful to use qualitative terms to report the results of the analysis, rather than quantitative values. The risk calculation may be completed quantitatively or by the use of qualitative terms.

A semi quantitative analysis (where the likelihood is linked to an indicative probability) or a qualitative analysis may be used:

- As an initial screening process to identify hazards and risks which require more detailed consideration and analysis.
- When the level of risk does not justify the time and effort required for more detailed analysis.
- Where the possibility of obtaining numerical data is limited such that a quantitative analysis is unlikely to be meaningful or may be misleading.

Section 7.3 describes a suitable and preferred terminology.

7.3 RISK MATRIX FOR PROPERTY LOSS

a) Adopt a defined qualitative terminology for likelihood, consequence and risk.

Qualitative terminology is presented in Appendix C for property loss. The terminology has been developed from Appendix G in AGS (2000) taking into account the experience and comments as discussed in the Commentary.

For ease of use, the frequency estimate, expressed as an annualized probability and taking into account the probability of spatial impact, is expressed qualitatively as likelihood.

The terminology is aimed primarily at residential development but may also be used for other situations. It is noted that provision of specific numerical values at the Notional Boundaries for the terms adopted does not reduce the uncertainty that may be associated with assessment of appropriate numerical values.

Where sufficient data is available, the risk should be determined from a quantitative analysis. The results can then be objectively compared, especially with quantified allowable risk criteria.

Where there is insufficient data or the study is at a walk over or preliminary design level, then use of qualitative methods or terms may be more appropriate. Use of risk ranking schemes, where component inputs are assigned relative ranks, may be suitable for initial screening. In other cases, it is likely that expression of the likelihood, consequence and risk using qualitative terms is preferable for communication purposes; (for example using terminology as in Appendix C). Selection of the appropriate term should be based on an appropriate evaluation of likelihood or consequence ranges.

Semi-quantitative methods may be a combination of both, for example considering risk to property qualitatively, and risk to life quantitatively based on the appropriate best estimates of likelihood.

b) The practitioner should adopt the preferred risk matrix presented in Appendix C.

The terminology presented in Appendix C of this Practice Note has addressed the shortcomings identified with the scheme in Appendix G AGS (2000). Appendix G of AGS (2000) is now superseded and should no longer be used. Adoption of Appendix C as a preferred risk matrix will assist with uniformity of assessment and interpretation. This is discussed further in the Commentary.

The regulator should only accept non standard schemes where the terms have been clearly defined, the terms have been explained in relation to the preferred terminology, and it can be reasonably demonstrated by the practitioner that the alternative is better suited to the particular circumstances of the assessment.

7.4 ESTIMATION OF RISK OF LOSS OF LIFE

a) Estimate the risk of loss of life quantitatively for the person most at risk.

The annual probability of loss of life for the person most at risk from the landslide(s) should be estimated using the equations in Section 7.1. The person most at risk will often but not always be the person with the greatest spatial temporal probability.

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The individual risk, as determined by summing the risk, for the person most at risk, from all the landslide hazards, is used for comparison with the tolerable risk criteria.

b) For situations where there is a potential for large numbers of lives to be lost in a single landslide event, estimate the frequency (f) –number (N) of lives lost pairs and total annual risk.

If the possible loss of large numbers of lives from a landslide incident is high, society will generally expect that the probability that the incident might actually occur should be low. This accounts for society's particular intolerance to incidents that cause many simultaneous casualties and is embodied in the criteria for tolerable societal risk. Societal Risk is discussed further in the Commentary.

In many cases there will be more than one landslide hazard (e.g. rockfall, which may lead to one or two lives lost; medium volume rapid landslide which may lead to several lives lost; and large rapid landslide which may lead to many lives lost). The frequency (annual probability, "f") of the "event" and the number of lives lost (N) should be estimated for each landslide hazard.

The total annual risk = $\sum (f \times N)$ should also be estimated.

8 RISK ASSESSMENT

8.1 RISK EVALUATION

Evaluate the risks against Tolerable Risk Criteria for loss of life and property loss.

Accept the risks if tolerable, or seek to reduce risks to tolerable levels by risk mitigation.

The main objectives of risk evaluation are usually to decide whether to accept or treat the risks and to set priorities. The Tolerable Risk Criteria are usually imposed by the regulator, unless agreed otherwise with the owner/client

Non- technical clients may seek guidance from the practitioner on whether to accept the risk. In these situations, risk comparisons, discussion of treatment options and explanation of the risk management process can help the client make his decision.

It is desirable, if not essential, that the practitioner who prepared the risk assessment be involved in the decision making process because the process is often iterative, requiring assessment of the sensitivity of calculations to assumptions, modification of the development proposed and revision of risk mitigation measures.

Risk evaluation involves making judgements about the significance and tolerability of the estimated risk. Evaluation may involve comparison of the assessed risks with other risks or with risk acceptance criteria related to finance, loss of life or other values. Risk evaluation may include consideration of issues such as environmental effects, public reaction, politics, business or public confidence and fear of litigation.

In a simple situation where the client/owner is the only affected party, risk evaluation may be a simple value judgement. In more complex situations, value judgements on acceptable risk appropriate to the particular situation are still made as part of an acceptable process of risk management.

8.2 TOLERABLE RISK CRITERIA

The regulator is to establish the Tolerable Risk Criteria for loss of life and property loss.

As discussed in Section 3.5, the regulator is the appropriate authority to set standards for tolerable risk which may relate not only to perceived safety in relation to other risks, but also to government policy. Implementation of a tolerable risk level has implications to the community at large, both in terms of relative risks or safety and in terms of economic impact on the community.

The Commentary provides discussion and gives the AGS recommendations in relation to tolerable risk for loss of life. These are summarized in Table 1

Table 1: AGS Suggested Tolerable loss of life individual risk.

Situation	Suggested Tolerable Loss of Life Risk for the person most at risk
Existing Slope (1) / Existing Development (2)	10^{-4} / annum
New Constructed Slope (3) / New Development (4) / Existing Landslide (5)	10^{-5} / annum

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Notes:

1. “Existing Slopes” in this context are slopes that are not part of a recognizable landslide and have demonstrated non-failure performance over at least several seasons or events of extended adverse weather, usually being a period of at least 10 to 20 years.
2. “Existing Development” includes existing structures, and slopes that have been modified by cut and fill, that are not located on or part of a recognizable landslide and have demonstrated non-failure performance over at least several seasons or events of extended adverse weather, usually being a period of at least 10 to 20 years.
3. “New Constructed Slope” includes any change to existing slopes by cut or fill or changes to existing slopes by new stabilisation works (including replacement of existing retaining walls or replacement of existing stabilisation measures, such as rock bolts or catch fences).
4. “New Development” includes any new structure or change to an existing slope or structure. Where changes to an existing structure or slope result in any cut or fill of less than 1.0m vertical height from the toe to the crest and this change does not increase the risk, then the Existing Slope / Existing Structure criterion may be adopted. Where changes to an existing structure do not increase the building footprint or do not result in an overall change in footing loads, then the Existing Development criterion may be adopted.
5. “Existing Landslides” have been considered likely to require remedial works and hence would become a New Constructed Slope and require the lower risk. Even where remedial works are not required per se, it would be reasonable expectation of the public for a known landslide to be assessed to the lower risk category as a matter of “public safety”.

Acceptable risks are usually considered to be one order of magnitude lower than the Tolerable Risks.

It is important to distinguish between “acceptable risks” and “tolerable risks”.

Tolerable Risks are risks within a range that society can live with so as to secure certain benefits. It is a range of risk regarded as non-negligible and needing to be kept under review and reduced further if practicable.

Acceptable Risks are risks which everyone affected is prepared to accept. Action to further reduce such risk is usually not required unless reasonably practicable measures are available at low cost in terms of money, time and effort.

AGS suggests that for most development in existing urban area criteria based on Tolerable Risks levels are applicable because of the trade-off between the risks, the benefits of development and the cost of risk mitigation.

The Commentary discusses Individual and Societal risk to loss of life. Usually Societal risk need not be considered for a risk evaluation in relation to a single dwelling. Societal risk should be evaluated for buildings having high numbers of occupants, such as schools, hospitals, hotels or motels where many lives are at risk. This then addresses society’s aversion to loss of many lives from single landslide events.

The Tolerable Risk Criteria for property loss may be determined by the Importance Level of the development (Appendix A) as discussed in the Commentary.

9 RISK MANAGEMENT

9.1 RISK MITIGATION PRINCIPLES

9.1.1 Feasible options for risk mitigation for each risk assessment are to be identified and discussed including the reduced risk by adoption of those options.

Alternative methods to be explored include:

- a. **Accept the risk**, which is only an option subject to the criteria set by the regulator. Where the risk is not tolerable then risk mitigation measures are required.
- b. **Avoid the risk**, such as relocation of the site of proposed development, or revise the form of the development, or abandon the development (though this may still require some risks to be controlled due to possible effect on third parties adjacent or nearby).
- c. **Reduce the frequency of landsliding**, by stabilisation measures to control the initiating circumstances, such as by re-profiling the surface geometry where existing slopes are ‘over steep’, by provision of improved surface water drainage measures, by provision of subsurface drainage scheme, by provision of retaining structures such as retaining walls, anchored walls or ground anchors.
- d. **Reduce the consequences**, by provision of defensive stabilisation measures or protective measures such as a boulder catch fence, or amelioration of the behaviour of the landslide, or by relocation of the development to a more favourable location.

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- e. **Manage the risk by establishing monitoring and warning systems**, such as by regular site visits, or by survey, which enable the risks to be managed as an interim measure in the short term or as a permanent measure for the long term by alerting persons potentially affected to a change in the landslide condition. Such systems may be regarded as a method of reducing the consequences provided it is feasible for sufficient time to be available between the alert being raised and appropriate action being implemented.
- f. **Transfer the risk**, such as by requiring another authority to accept the risk (possibly via a court appraisal) or by provision of insurance to cover potential property damage.
- g. **Postpone the decision**, where there is sufficient uncertainty resulting from the available data, provided that additional investigations or monitoring are likely to enable a better risk assessment to be completed. Postponement is only a temporary measure and implies the risks are being temporarily accepted, even though they may not be acceptable or tolerable.

Adoption of particular risk mitigation measures needs to be documented so that the decisions are transparent to future land owners and to the regulator. The documentation will need to make it clear whether there is ongoing maintenance required or not. Responsibility for implementation of the risk mitigation measures (including auditing and reporting) resides with the land owner, particularly where ongoing maintenance is required.

It should be recognized that there may be situations where the risk is such that either no development should occur, or that very strict conditions and/or extensive investigations and implementation of risk control measures will be required. Such risk control measures may render the proposed development unworkable.

9.1.2 Wherever possible the recommended options should be engineered to reduce the uncertainties.

It is not possible to remove risk, but it can be reduced.

Risk mitigation options should include robust engineering design to reduce uncertainties and hence the risk.

Guidance on good engineering practice for hillside design and construction is given in Appendix G which has been reproduced from AGS (2000).

It is necessary that the options considered lower the risk to at least tolerable levels. In many cases, the ALARP principle (“As Low As Reasonably Practicable” as discussed in the Commentary) may apply so that reduction to a tolerable level is a pragmatic result since reduction to acceptable levels is not viable in the context of the cost to the individual or community. In other cases, good practice may suggest that risk reduction be applied since it is relatively cheap or cost effective to implement even though risk levels are assessed to already be at acceptable levels. In other words, risk minimization should be a governing feature or tenet of LRM.

Evaluation of mitigation options may take into account relative costs and effectiveness of the measures and inherent uncertainties. Combinations of mitigation measures may be appropriate.

The options should be reassessed if there is a need to reduce uncertainties or if suitable engineering options cannot be adopted.

An issue will be who decides on what level of risk reduction is appropriate. This is dependent on the risk tolerance criteria set by the regulator. The owner is likely to input into selection of the options, subject to approvals by the regulator. For some cases, there may be discussion between the stakeholders to select a suitable scheme of risk mitigation measures.

9.1.3 The adopted risk mitigation measures are to be detailed in a mitigation plan to explain and document the implementation of the measures.

The mitigation plan should identify responsibilities for each stakeholder during and after implementation. It may also include cost estimates, programme, required inspection regime, performance measures and expected outcomes. The level of detail will depend on the priority for the option and stage of the evaluation and implementation process.

The mitigation plan may include an emergency plan which should establish from the outset the sequence of events or monitoring results that will activate this plan. The plan may include a number of warning levels and consequent actions. The plan must be carefully reviewed to confirm it is workable and will achieve the desired risk mitigation.

The existence of the mitigation plan needs to be readily known to subsequent land owners. The most readily available method for this is to register the mitigation plan details on the land title.

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9.1.4 The risk should be subject to monitoring and review during the assessment of options, during implementation of the risk mitigation measures and during the on going monitoring.

Further data may come to light during the management process which enables the risks to be reassessed. Such data may be adverse, requiring more stringent risk mitigation measures, or alternatively may be positive by demonstrating satisfactory slope performance under adverse conditions. It is anticipated that the practitioner would have a primary role in the monitoring and review process and particularly to confirm the requirements of the approval conditions had been fulfilled.

9.2 SITE SPECIFIC DEVELOPMENT CONDITIONS

Identify appropriate site specific development conditions to provide good practice and control the risks to acceptable levels.

In the context of advice from a technical expert (the practitioner) acting in a consultant capacity, development controls would usually constitute 'recommendations', but as they will be integral with the risk assessment of the final development they may not be optional to the client. The practitioner should provide a statement as to the appropriateness of the development proposals in relation to the risk management requirements.

If 'certification' of the completed development is required (by the planning scheme or regulator's approval conditions), then the development conditions and associated inspections and documentation must be sufficient to enable this to be provided at the later date.

The development conditions should be subdivided into those required at each of the stages of detailed design, construction (including appropriate sequencing and temporary works), and for maintenance. The development conditions must address all the factors relevant to controlling the landslide risk.

9.3 DESIGN LIFE

9.3.1 Design of the risk mitigation measures is to be suitable for the time frame of the life of the structure - the design life. The design life is to be clearly stated on the design drawings.

Often the design life will be that specified by relevant design codes such as 40 to 60 years for AS3600 Concrete Code, 50 years for AS2870 Residential Slabs and Footings, or for 5 years to 120 years for temporary site works to major public works respectively for AS4678 Earth Retaining Structures.

A design life of at least 50 years would be considered to be reasonable for permanent structures used by people. Some local government policies may require a longer design life as discussed in the Commentary. However, for some structures, such as timber retaining walls, inherent performance of the materials will limit the effective performance life to less than the required design life.

9.3.2 Where the effective performance life is less than the required design life, then the effective life should be extended by a maintenance regime designed to overcome the limitations and to enable the performance to be assessed throughout the required design life. This is likely to require more extensive repair and replacement as determined by regular maintenance inspections.

For example, experience shows the longevity of timber crib walls is less than for a concrete structure, due to faster degradation of timber with time. Therefore, a more frequent inspection and maintenance / repair / replacement regime will be required for timber crib walls to enable suitable repair and replacement so that a reasonable design life can be achieved. Similar considerations will apply to subsoil drains and stressed anchors.

9.4 MAINTENANCE REQUIREMENTS

9.4.1 The design is to include details of required inspections and maintenance to enable the risk mitigation measures to remain effective for at least the design life of the structure.

Risk mitigation is not just an exercise in LRM documentation, design of the works and construction of the risk mitigation measures. The owner, including all owners subsequent to those responsible for commissioning the risk mitigation measures, has a responsibility to inspect and maintain the risk mitigation measures.

9.4.2 Refer to the AGS Australian GeoGuide LR111 which provides advice on record keeping.

The other GeoGuides (AGS, 2007e) also provide advice on the frequency of maintenance tasks.

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- 9.4.3 Implementation of the maintenance plan may require ‘enforcement’ by annotation on the land title so that subsequent purchasers become aware of the requirements and that relevant documents are available for the maintenance plan. Such ‘enforcement’ will be a benefit to subsequent owners as they will be better informed as to their required input responsibilities.**

10 REPORTING STANDARDS

- 10.1 The report on the risk assessment is to document the data gathered, the logic applied and conclusion reached in a defensible manner.**

The practitioner will gather relevant data, will assess the relevance of the data and will reach conclusions as to the appropriate geotechnical model and basic assessment of the slope forming processes and rates. Full documentation of these results provides evidence of completion, provides transparency in the light of uncertainty, enables the assessment to be re-examined or extended at a later date and enables the assessment to be defended against critical review. The process often identifies uncertainties or limitations of the assessment which also need to be documented and understood.

- 10.2 The data to be presented includes:**

- a. List of data sources.
- b. Discussion of investigation methods used, and any limitations thereof.
- c. Site plan (to scale) with geomorphic mapping results.
- d. All factual data from investigations, such as borehole and test pit logs, laboratory test results, groundwater level observations, record photographs.
- e. Location of all subsurface investigations and/or outcrops/cuttings.
- f. Location of cross section(s).
- g. Cross section(s) (to scale) with interpreted subsurface model showing investigation locations.
- h. Evidence of past performance.
- i. Local history of instability with assessed trigger events.
- j. Identification of landslides, on plan or section or both, and discussed in terms of the geomorphic model, relevant slope forming process and process rates. Landslides need to be considered above the site, below the site and adjacent to the site.
- k. Assessed likelihood of each landslide with basis thereof.
- l. Assessed consequence to property and life for each landslide with basis thereof.
- m. Resulting risk for each landslide.
- n. Risk assessment in relation to tolerable risk criteria (e.g. regulator’s published criteria where appropriate).
- o. Risk mitigation measures and options, including reassessed risk once these measures are implemented.

Where any of the above is not or cannot be completed, the report should document the missing elements, including an explanation as to why.

The report needs to clearly state whether the risk assessment is based on existing conditions or with risk treatment measures implemented. In some cases, the assessment for both existing and after treatment should be documented to demonstrate the effect of risk control measures on reducing risk.

A report which does not properly document the assessment is of limited value and would appear to have no reasonable basis.

11 SPECIAL CHALLENGES

11.1 MINOR WORKS

Adoption of all the provisions of the Practice Note for minor works may not be appropriate or reasonable. However, the basic principles still need to be considered. Although some policies may make provision for less onerous consideration for minor works, the practitioner will still have a duty of care to advise on all aspects and may have other landslides not connected with the proposed works that will still need to be considered.

Minor works should be evaluated on a site by site basis but are likely to comprise proposed works of relatively low monetary value (such as may be completed by an owner builder with appropriate approvals and insurances) or those which do not change the existing risk, provided the existing risk has been assessed to be within the tolerable range. In some cases, the risk to life may be much higher than the risk to property and may dictate the need for risk mitigation to achieve tolerable risk levels.

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11.2 PART OF THE SITE NOT ACCEPTABLE

Existing or proposed development may not involve the full site area. Nonetheless, the practitioner's report must address all risks and advise the client and/or regulator of necessary works to control risks on other parts of the site or adjacent/nearby sites upslope or down slope as appropriate (as a primary duty of care issue).

Where additional development is proposed, it may be found that risks associated with the proposed development are tolerable but that landslide risks on other parts of the site are not. These other risks still must be addressed.

11.3 ADJOINING AREAS NOT UNDER RESPONSIBILITY OF THE SITE OWNER

In some cases, the risk posed by landslides in areas beyond the control of the land owner may be intolerable.

The LRM assessment report must identify these landslides and provide a preliminary assessment of appropriate risk mitigation measures, which may require further investigation to better assess the risk.

The regulator may then implement appropriate orders (as appropriate to the legal/regulatory framework) to enforce appropriate risk mitigation measures and/or investigations. Alternatively, it may not be appropriate for development to proceed in such cases.

11.4 COASTAL CLIFFS

LRM reports on coastal cliffs should include consideration of the existing slope profile, evidence of past instability, geology, defects, ground water, degradation cycles, and degradation rates and possible effects of wave attack, wave run-up and sea spray. The cliff areas should be examined from the face side as well as from the land side.

Assessment of coastal cliffs is likely to require special expertise to consider the combined effects associated with recession rates, rock mechanics and wave environment. The LRM assessment may require some input from coastal engineers to address possible effects from storm events in terms of wave heights, run-up and frequency. The most frequent hazard is often boulder falls which will have risk determined by the temporal spatial probability.

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13 REFERENCES

- AGS (2000) Australian Geomechanics Society “*Landslide Risk Management Concepts and Guidelines*” Australian Geomechanics, Vol35 No1 March 2000 pp49-92, and reprinted in Vol37 No2 May 2002
- AGS (2007a) “Guideline for Landslide Susceptibility, Hazard and Risk Zoning for Land Use Planning” Australian Geomechanics Society, Australian Geomechanics Vol42 No1 March 2007
- AGS (2007d) “Commentary on the Practice Note Guidelines for Landslide Risk Management” Australian Geomechanics Society, Australian Geomechanics Vol 42 No1 March 2007
- AGS (2007e), “*Australian GeoGuides for Slope Management and Maintenance*” Australian Geomechanics Society, Australian Geomechanics Vol 42 No1 March 2007
- ANCOLD (2003) “*Guidelines on Risk Assessment*” Australian National Committee on Large Dams Inc
- BCA Building Code of Australia, Australian Building Codes Board
- Corominas, J. (1996) The angle of reach as a mobility index for small and large landslides. Canadian Geotechnical Journal 33: 260-271
- Fell, R., Ho, K.K.S., Lacasse, S., Leroi, E. (2005) “*A framework for landslide risk assessment and management*” Proc Intl Conf on Landslide Risk Management, Vancouver, Canada, 31 May – 3 June 2005, Eds Hungr, Fell, Couture & Eberhardt, AA Balkema. 3 - 25
- Hunter, G. and Fell, R. (2002) “Estimation of Travel Distance for Landslides in Soil Slopes” Australian Geomechanics Vol37 No2 May, 65 - 82
- Khalili, N., Fell, R. and Tai, K.S. (1996). A simplified method for estimating failure induced deformation. Proc. Seventh Int. Symp. On Landslides. Editor Senesett, K., Balkema, Rotterdam, 1263-1268.
- Lee, E.M. and Jones, D.K.C. (2004) “Landslide Risk Assessment” Thomas Telford, 454p.
- Moon, A.T. and Wilson, R.A. (2004) “*Will it happen? Quantitative judgments of landslide likelihood*”, Proc 9th Australia New Zealand Conference on Geomechanics, “*To the eNZ of the Earth*”, New Zealand Geotechnical Society & Australian Geomechanics Society, University of Auckland Centre for Continuing Education publ., G Farquhar, P Kesley, J March, D Fellows eds, Vol 2, pp754
- Moon, A.T., Wilson, R.A. and Flentje, P.N. (2005) “*Developing and using landslide size frequency models*”, Proc Int Conf on Landslide Risk Management, Vancouver, 31 May-3 June 2005, AA Balkema Publ, O. Hungr, R. Fell, R. Couture & E. Eberhardt eds., pp681.

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

- Picarelli, L., Oboni, F., Evans, S.G., Mostyn, G. and Fell, R., (2005) "*Hazard characterization and quantification*" Proc Int Conf on Landslide Risk Management, Vancouver, 31 May-3 June 2005, AA Balkema Publ, O. Hungr, R. Fell, R. Couture and E. Eberhardt eds., pp681
- Varnes, D.J. and The International Association of Engineering Geology Commission on Landslides and other Mass Movements (1984). *Landslide Hazard Zonation: A review of principles and practice*. Natural Hazards, Vol 3, Paris, France. UNESCO, 63p.
- Standards Australia (1996) "*Residential Slabs and Footings*" Australian Standard AS2870
- Standards Australia (2001) "*Concrete Structures*" Australian Standard AS3600
- Standards Australia (2001) "*Steel Structures*" Australian Standard AS4100
- Standards Australia (2002) "*Earth Retaining Structures*" Australian Standard AS4678.

APPENDIX A - DEFINITION OF TERMS AND LANDSLIDE RISK

RISK TERMINOLOGY

Acceptable Risk – A risk for which, for the purposes of life or work, we are prepared to accept as it is with no regard to its management. Society does not generally consider expenditure in further reducing such risks justifiable.

Annual Exceedance Probability (AEP) – The estimated probability that an event of specified magnitude will be exceeded in any year.

Consequence – The outcomes or potential outcomes arising from the occurrence of a landslide expressed qualitatively or quantitatively, in terms of loss, disadvantage or gain, damage, injury or loss of life.

Elements at Risk – The population, buildings and engineering works, economic activities, public services utilities, infrastructure and environmental features in the area potentially affected by landslides.

Frequency – A measure of likelihood expressed as the number of occurrences of an event in a given time. See also Likelihood and Probability.

Hazard – A condition with the potential for causing an undesirable consequence (the landslide). The description of landslide hazard should include the location, volume (or area), classification and velocity of the potential landslides and any resultant detached material, and the likelihood of their occurrence within a given period of time.

Individual Risk to Life – The risk of fatality or injury to any identifiable (named) individual who lives within the zone impacted by the landslide; or who follows a particular pattern of life that might subject him or her to the consequences of the landslide.

Landslide Activity – The stage of development of a landslide; pre failure when the slope is strained throughout but is essentially intact; failure characterised by the formation of a continuous surface of rupture; post failure which includes movement from just after failure to when it essentially stops; and reactivation when the slope slides along one or several pre-existing surfaces of rupture. Reactivation may be occasional (eg seasonal) or continuous (in which case the slide is "active").

Landslide Intensity – A set of spatially distributed parameters related to the destructive power of a landslide. The parameters may be described quantitatively or qualitatively and may include maximum movement velocity, total displacement, differential displacement, depth of the moving mass, peak discharge per unit width, kinetic energy per unit area.

Landslide Risk - The AGS Australian GeoGuide LR7 (AGS, 2007e) should be referred to for an explanation of Landslide Risk.

Landslide Susceptibility – The classification, and volume (or area) of landslides which exist or potentially may occur in an area or may travel or retrogress onto it. Susceptibility may also include a description of the velocity and intensity of the existing or potential landsliding.

Likelihood – Used as a qualitative description of probability or frequency.

Probability – A measure of the degree of certainty. This measure has a value between zero (impossibility) and 1.0 (certainty). It is an estimate of the likelihood of the magnitude of the uncertain quantity, or the likelihood of the occurrence of the uncertain future event.

There are two main interpretations:

- (i) Statistical – frequency or fraction – The outcome of a repetitive experiment of some kind like flipping coins. It includes also the idea of population variability. Such a number is called an "objective" or relative frequentist probability because it exists in the real world and is in principle measurable by doing the experiment.
- (ii) Subjective probability (degree of belief) – Quantified measure of belief, judgment, or confidence in the likelihood of an outcome, obtained by considering all available information honestly, fairly, and with a minimum of

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bias. Subjective probability is affected by the state of understanding of a process, judgment regarding an evaluation, or the quality and quantity of information. It may change over time as the state of knowledge changes.

Qualitative Risk Analysis – An analysis which uses word form, descriptive or numeric rating scales to describe the magnitude of potential consequences and the likelihood that those consequences will occur.

Quantitative Risk Analysis – An analysis based on numerical values of the probability, vulnerability and consequences and resulting in a numerical value of the risk.

Risk – A measure of the probability and severity of an adverse effect to health, property or the environment. Risk is often estimated by the product of probability x consequences. However, a more general interpretation of risk involves a comparison of the probability and consequences in a non-product form.

Risk Analysis – The use of available information to estimate the risk to individual, population, property, or the environment, from hazards. Risk analyses generally contain the following steps: Scope definition, hazard identification and risk estimation.

Risk Assessment – The process of risk analysis and risk evaluation.

Risk Control or **Risk Treatment** – The process of decision making for managing risk and the implementation or enforcement of risk mitigation measures and the re-evaluation of its effectiveness from time to time, using the results of risk assessment as one input.

Risk Estimation – The process used to produce a measure of the level of health, property or environmental risks being analysed. Risk estimation contains the following steps: frequency analysis, consequence analysis and their integration.

Risk Evaluation – The stage at which values and judgments enter the decision process, explicitly or implicitly, by including consideration of the importance of the estimated risks and the associated social, environmental and economic consequences, in order to identify a range of alternatives for managing the risks.

Risk Management – The complete process of risk assessment and risk control (or risk treatment).

Societal Risk – The risk of multiple fatalities or injuries in society as a whole: one where society would have to carry the burden of a landslide causing a number of deaths, injuries, financial, environmental and other losses.

Susceptibility – see **Landslide Susceptibility**

Temporal Spatial Probability – The probability that the element at risk is in the area affected by the landsliding, at the time of the landslide.

Tolerable Risk – A risk within a range that society can live with so as to secure certain net benefits. It is a range of risk regarded as non-negligible and needing to be kept under review and reduced further if possible.

Vulnerability – The degree of loss to a given element or set of elements within the area affected by the landslide hazard. It is expressed on a scale of 0 (no loss) to 1 (total loss). For property, the loss will be the value of the damage relative to the value of the property; for persons, it will be the probability that a particular life (the element at risk) will be lost, given the person(s) is affected by the landslide.

ASSOCIATED TERMINOLOGY

Importance Level – of a building or structure is directly related to the societal requirements for its use, particularly during or following extreme events. The consequences with respect to life safety of the occupants of buildings are indirectly related to the Importance Level, being a result of the societal requirement for the structure rather than the reason *per se* of the Importance Level.

Authority or **Council** having statutory responsibility for community activities, community safety and development approval or management of development within its defined area/region.

The **Regulator** will be the responsible body/authority for setting Acceptable/Tolerable Risk Criteria to be adopted for the community/region/activity, which will be the basis for setting levels for Acceptable and Tolerable Risk in the application of the risk assessment guidelines.

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Importance Level of Structure	Explanation	Examples (Regulatory authorities may designate any structure to any classification type when local conditions make such desirable)
1	Buildings or structures generally presenting a low risk to life and property (including other property).	Farm buildings. Isolated minor storage facilities. Minor temporary facilities. Towers in rural situations.
2	Buildings and structures not covered by Importance Levels 1, 3 or 4.	Low-rise residential construction. Buildings and facilities below the limits set for Importance Level 3.
3	Buildings or structures that as a whole may contain people in crowds, or contents of high value to the community, or that pose hazards to people in crowds.	Buildings and facilities where more than 300 people can congregate in one area. Buildings and facilities with primary school, secondary school or day-care facilities with capacity greater than 250. Buildings and facilities for colleges or adult education facilities with a capacity greater than 500. Health care facilities with a capacity of 50 or more residents but not having surgery or emergency treatment facilities. Jails and detention facilities. Any occupancy with an occupant load greater than 5,000. Power generating facilities, water treatment and waste water treatment facilities, any other public utilities not included in Importance Level 4. Buildings and facilities not included in Importance Level 4 containing hazardous materials capable of causing hazardous conditions that do not extend beyond property boundaries.
4	Buildings or structures that are essential to post-disaster recovery, or with significant post-disaster functions, or that contain hazardous materials.	Buildings and facilities designated as essential facilities. Buildings and facilities with special post-disaster functions. Medical emergency or surgery facilities. Emergency service facilities: fire, rescue, police station and emergency vehicle garages. Utilities required as back-up for buildings and facilities of Importance Level 4. Designated emergency shelters. Designated emergency centres and ancillary facilities. Buildings and facilities containing hazardous (toxic or explosive) materials in sufficient quantities capable of causing hazardous conditions that extend beyond property boundaries.

(from BCA Guidelines)

Practitioner – A specialist Geotechnical Engineer or Engineering Geologist who is degree qualified, is a member of a professional institute and who has achieved chartered professional status – being either Chartered Professional Engineer (CPEng) within the Institution of Engineers Australia, Chartered Professional Geologist (CPGeo) within the Australasian Institute of Mining & Metallurgy, or Registered Professional Geoscientist (RPGeo) within the Australian Institute of Geoscientists – specifically with Landslide Risk Management as a core competency.

A Practitioner will include persons qualified under the Institution of Engineers Australia NPER – LRM register.

It would normally be required that the Practitioner can demonstrate an appropriate minimum period of experience in the practice of landslide risk assessment and management in the geographic region, or can demonstrate relevant experience in similar geological settings.

Regulator – The regulatory authority [Federal Government/ State Government/ Instrumentality/ Regional/Local.

APPENDIX B - LANDSLIDE TERMINOLOGY

The following provides a summary of landslide terminology which should (for uniformity of practice) be adopted when classifying and describing a landslide. It has been based on Cruden & Varnes (1996) and the reader is recommended to refer to the original documents for a more detailed discussion, other terminology and further examples of landslide types and processes.

Landslide

The term **landslide** denotes “the movement of a mass of rock, debris or earth down a slope”. The phenomena described as landslides are not limited to either the “land” or to “sliding”, and usage of the word has implied a much more extensive meaning than its component parts suggest. Ground subsidence and collapse are excluded.

Classification of Landslides

Landslide classification is based on Varnes (1978) system which has two terms: the first term describes the material type and the second term describes the type of movement.

The material types are **Rock**, **Earth** and **Debris**, being classified as follows:-

The material is either rock or soil.

- Rock:** is “a hard or firm mass that was intact and in its natural place before the initiation of movement.”
- Soil:** is “an aggregate of solid particles, generally of minerals and rocks, that either was transported or was formed by the weathering of rock in place. Gases or liquids filling the pores of the soil form part of the soil.”
- Earth:** “describes material in which 80% or more of the particles are smaller than 2 mm, the upper limit of sand sized particles.”
- Debris:** “contains a significant proportion of coarse material; 20% to 80% of the particles are larger than 2 mm and the remainder are less than 2 mm.”

The terms used should describe the displaced material in the landslide before it was displaced.

The types of movement describe how the landslide movement is distributed through the displaced mass. The five kinematically distinct types of movement are described in the sequence **fall**, **topple**, **slide**, **spread** and **flow**.

The following table shows how the two terms are combined to give the landslide type:

Table B1: Major types of landslides. Abbreviated version of Varnes’ classification of slope movements (Varnes, 1978).

TYPE OF MOVEMENT		TYPE OF MATERIAL	
		BEDROCK	ENGINEERING SOILS
			Predominantly Coarse Predominantly Fine
FALLS		Rock fall	Debris fall Earth fall
TOPPLES		Rock topple	Debris topple Earth topple
SLIDES	ROTATIONAL	Rock slide	Debris slide Earth slide
	TRANSLATIONAL		
LATERAL SPREADS		Rock spread	Debris spread Earth spread
FLOWS		Rock flow (Deep creep)	Debris flow Earth flow (Soil creep)
COMPLEX		Combination of two or more principle types of movement	

Figure B1 gives schematics to illustrate the major types of landslide movement. Further information and photographs of landslides are available on the USGS website at <http://landslides.usgs.gov>.

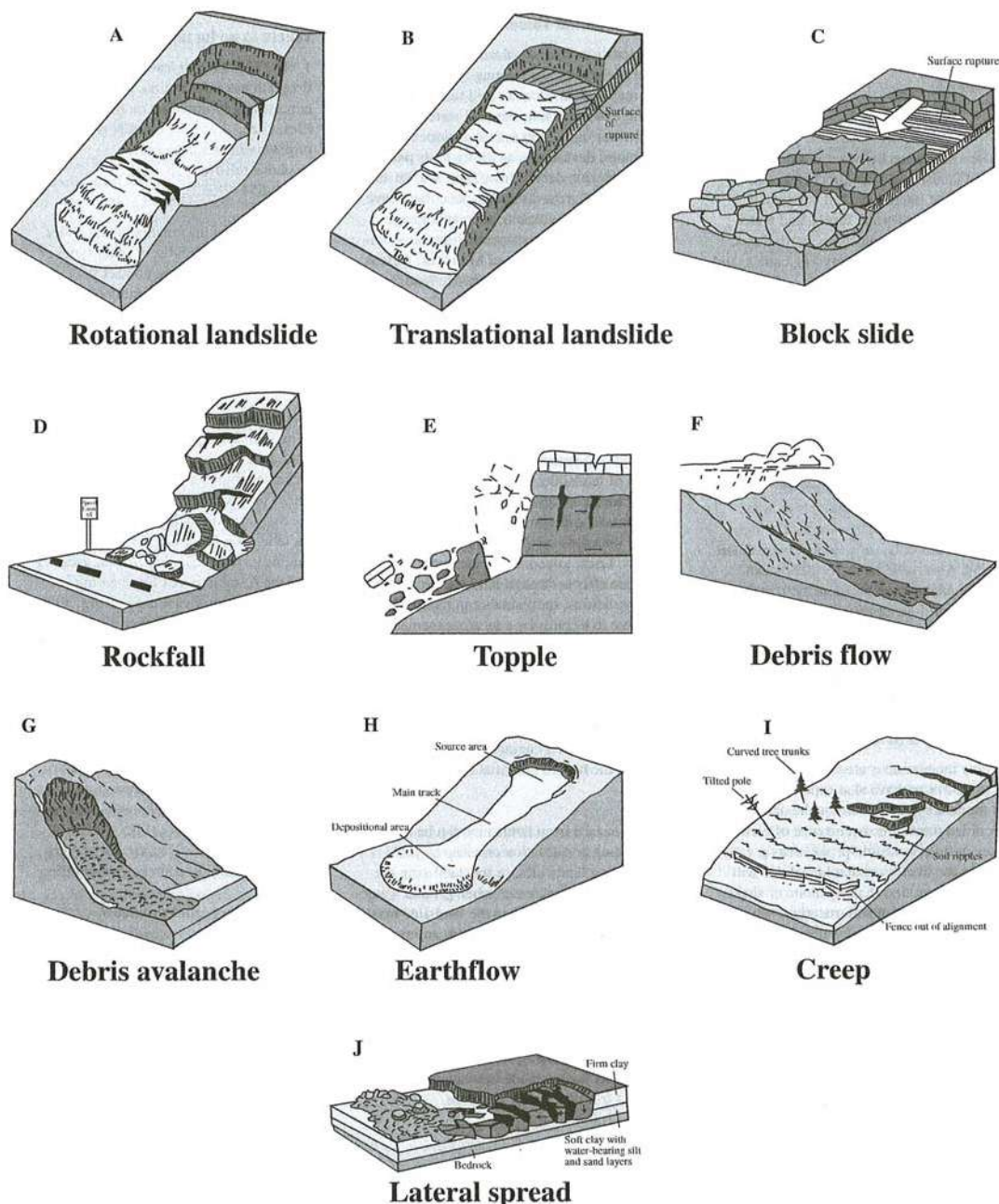


Figure B1: These schematics illustrate the major types of landslide movement.
(From US Geological Survey Fact Sheet 2004-3072, July 2004, with kind permission for reproduction.)

The nomenclature of a landslide can become more elaborate as more information about the movement becomes available. To build up the complete identification of the movement, descriptors are added in front of the two-term classification using a preferred sequence of terms. The suggested sequence provides a progressive narrowing of the focus of the descriptors, first by time and then by spatial location, beginning with a view of the whole landslide, continuing with parts of the movement and finally defining the materials involved. The recommended sequence, as shown in Table B2, describes activity (including state, distribution and style) followed by descriptions of all movements (including rate, water content, material and type). Definitions of the terms in Table B2 are given in Cruden & Varnes (1996).

Second or subsequent movements in complex or composite landslides can be described by repeating, as many times as necessary, the descriptors used in Table B2. Descriptors that are the same as those for the first movement may then be dropped from the name.

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For example, the very large and rapid slope movement that occurred near the town of Frank, Alberta, Canada, in 1903 was a *complex, extremely rapid, dry rock fall – debris flow*. From the full name of this landslide at Frank, one would know that both the debris flow and the rock fall were extremely rapid and dry because no other descriptors are used for the debris flow.

The full name of the landslide need only be given once; subsequent references should then be to the initial material and type of movement; for the above example, “the rock fall” or “the Frank rock fall” for the landslide at Frank, Alberta.

Table B2: Glossary for forming names of landslides.

Activity			
State	Distribution	Style	
Active	Advancing	Complex	
Reactivated	Retrogressive	Composite	
Suspended	Widening	Multiple	
Inactive	Enlarging	Successive	
Dormant	Confined	Single	
Abandoned	Diminishing		
Stabilised	Moving		
Relict			
Description of First Movement			
Rate	Water Content	Material	Type
Extremely rapid	Dry	Rock	Fall
Very rapid	Moist	Earth	Topple
Rapid	Wet	Debris	Slide
Moderate			Spread
Slow			Flow
Very slow			
Extremely slow			

Note: Subsequent movements may be described by repeating the above descriptors as many times as necessary. These terms are described in more detail in Cruden & Varnes (1996) and examples are given.

Landslide Features

Varnes (1978, Figure 2.1t) provided an idealised diagram showing the features for a *complex earth slide – earth flow*, which has been reproduced here as Figure B2. Definitions of landslide dimensions are given in Cruden & Varnes (1996).

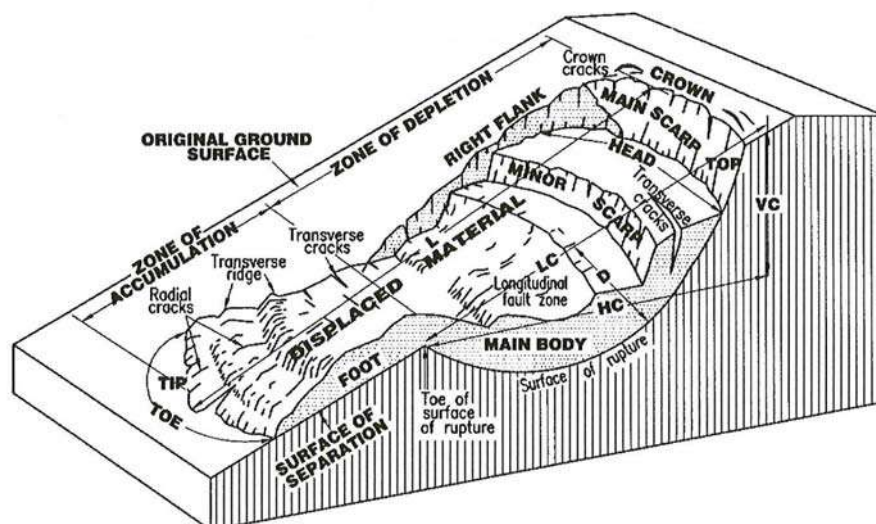


Figure B2: Block of Idealised Complex Earth Slide – Earth Flow

(Varnes, D J (1978), *Slope Movement Types and Processes*. In *Special Report 176: Landslides: Analysis and Control* (R L Schuster & R J Krizek, eds.), TRB, National Research Council, Washington, DC, pp.11-33).

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Rate of Movement

Figure B3 shows the velocity scale proposed by Cruden & Varnes (1996) which rationalises previous scales. The term “creep” has been omitted due to the many definitions and interpretations in the literature.

Velocity Class	Description	Velocity (mm/sec)	Typical Velocity	Probable Destructive Significance
7	Extremely Rapid			Catastrophe of major violence; buildings destroyed by impact of displaced material; many deaths; escape unlikely
		5×10^3	5 m/sec	
6	Very Rapid			Some lives lost; velocity too great to permit all persons to escape
		5×10^1	3 m/min	
5	Rapid			Escape evaluation possible; structures, possessions, and equipment destroyed
		5×10^{-1}	1.8 m/hr	
4	Moderate			Some temporary and insensitive structures can be temporarily maintained
		5×10^{-3}	13 m/month	
3	Slow			Remedial construction can be undertaken during movement; insensitive structures can be maintained with frequent maintenance work if total movement is not large during a particular acceleration phase
		5×10^{-5}	1.6 m/year	
2	Very Slow			Some permanent structures undamaged by movement
		5×10^{-7}	15 mm/year	
	Extremely SLOW			Imperceptible without instruments; construction POSSIBLE WITH PRECAUTIONS

Figure B3: Proposed Landslide Velocity Scale and Probable Destructive Significance.

REFERENCES AND ACKNOWLEDGEMENT

- Cruden, D.M., & Varnes, D.J. (1996), “Landslide Types and Processes”, Ch.3 in “Landslides. Investigation and Mitigation”, Eds Turner, A.K. and Schuster, R.L. Special Report 247, Transport Research Board, National Research Council, Washington D.C. Extracts reprinted above by kind permission of the authors and publishers. Copies of the publication can be obtained from “Transport Research Board, National Research Council, 2101 Constitution Avenue, N.W., Washington D.C. 20418, USA.
- IAEG (International Association of Engineering Geology) Commission on Landslides, (1990). Suggested nomenclature for landslides, Bulletin IAEG, No. 41, pp.13-16.
- Varnes, D.J. (1978). Slope Movement Types and Processes. In *Special Report 176: Landslides: Analysis and Control* (R.L. Schuster and R.J. Krizek, eds.), TRB, National Research Council, Washington, D.C., pp.11-33.
- WP/WLI (International Geotechnical Societies’ UNESCO Working Party on World Landslide Inventory) (1990). A suggested method for reporting a landslide. Bulletin IAEG, 41, pp.5-12
- WP/WLI (International Geotechnical Societies’ UNESCO Working Party on World Landslide Inventory) (1993). A suggested method for describing the activity of a landslide. Bulletin International Association of Engineering Geology, 47: 53-57.
- WP/WLI (International Geotechnical Societies’ UNESCO Working Party on World Landslide Inventory) (1994). Multilingual Glossary for Landslides, Bitech Press, Vancouver, in press.

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007
APPENDIX C: LANDSLIDE RISK ASSESSMENT
QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY

QUALITATIVE MEASURES OF LIKELIHOOD

Approximate Annual Probability		Implied Indicative Landslide Recurrence Interval		Description	Descriptor	Level
Indicative Value	Notional Boundary					
10 ⁻¹	5x10 ⁻²	10 years	20 years	The event is expected to occur over the design life.	ALMOST CERTAIN	A
10 ⁻²		100 years		The event will probably occur under adverse conditions over the design life.	LIKELY	B
10 ⁻³	5x10 ⁻³	1000 years	200 years	The event could occur under adverse conditions over the design life.	POSSIBLE	C
10 ⁻⁴	5x10 ⁻⁴	10,000 years	2000 years	The event might occur under very adverse circumstances over the design life.	UNLIKELY	D
10 ⁻⁵	5x10 ⁻⁵	100,000 years	20,000 years	The event is conceivable but only under exceptional circumstances over the design life.	RARE	E
10 ⁻⁶	5x10 ⁻⁶	1,000,000 years	200,000 years	The event is inconceivable or fanciful over the design life.	BARELY CREDIBLE	F

Note: (1) The table should be used from left to right; use Approximate Annual Probability or Description to assign Descriptor, not *vice versa*.

QUALITATIVE MEASURES OF CONSEQUENCES TO PROPERTY

Approximate Cost of Damage		Description	Descriptor	Level
Indicative Value	Notional Boundary			
200%	100%	Structure(s) completely destroyed and/or large scale damage requiring major engineering works for stabilisation. Could cause at least one adjacent property major consequence damage.	CATASTROPHIC	1
60%		Extensive damage to most of structure, and/or extending beyond site boundaries requiring significant stabilisation works. Could cause at least one adjacent property medium consequence damage.	MAJOR	2
20%	40%	Moderate damage to some of structure, and/or significant part of site requiring large stabilisation works. Could cause at least one adjacent property minor consequence damage.	MEDIUM	3
5%	10%	Limited damage to part of structure, and/or part of site requiring some reinstatement stabilisation works.	MINOR	4
0.5%	1%	Little damage. (Note for high probability event (Almost Certain), this category may be subdivided at a notional boundary of 0.1%. See Risk Matrix.)	INSIGNIFICANT	5

- Notes:** (2) The Approximate Cost of Damage is expressed as a percentage of market value, being the cost of the improved value of the unaffected property which includes the land plus the unaffected structures.
- (3) The Approximate Cost is to be an estimate of the direct cost of the damage, such as the cost of reinstatement of the damaged portion of the property (land plus structures), stabilisation works required to render the site to tolerable risk level for the landslide which has occurred and professional design fees, and consequential costs such as legal fees, temporary accommodation. It does not include additional stabilisation works to address other landslides which may affect the property.
- (4) The table should be used from left to right; use Approximate Cost of Damage or Description to assign Descriptor, not *vice versa*

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX C: – QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY (CONTINUED)

QUALITATIVE RISK ANALYSIS MATRIX – LEVEL OF RISK TO PROPERTY

LIKELIHOOD		CONSEQUENCES TO PROPERTY (With Indicative Approximate Cost of Damage)				
	Indicative Value of Approximate Annual Probability	1: CATASTROPHIC 200%	2: MAJOR 60%	3: MEDIUM 20%	4: MINOR 5%	5: INSIGNIFICANT 0.5%
A – ALMOST CERTAIN	10 ⁻¹	VH	VH	VH	H	M or L (5)
B – LIKELY	10 ⁻²	VH	VH	H	M	L
C – POSSIBLE	10 ⁻³	VH	H	M	M	VL
D – UNLIKELY	10 ⁻⁴	H	M	L	L	VL
E – RARE	10 ⁻⁵	M	L	L	VL	VL
F – BARELY CREDIBLE	10 ⁻⁶	L	VL	VL	VL	VL

Notes: (5) For Cell A5, may be subdivided such that a consequence of less than 0.1% is Low Risk.

(6) When considering a risk assessment it must be clearly stated whether it is for existing conditions or with risk control measures which may not be implemented at the current time.

RISK LEVEL IMPLICATIONS

Risk Level		Example Implications (7)
VH	VERY HIGH RISK	Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of the property.
H	HIGH RISK	Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.
M	MODERATE RISK	May be tolerated in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.
L	LOW RISK	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance is required.
VL	VERY LOW RISK	Acceptable. Manage by normal slope maintenance procedures.

Note: (7) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide.

APPENDIX D -EXAMPLE FORMS

The following example forms have been prepared as templates to provide appropriate documentation for the control of submissions and approval process.

It is envisaged that the regulator would edit the forms to suit local requirements and to use terminology appropriate to regulatory framework of the regulator's LRM policy. Items between '<>' are to be edited as appropriate. The following terms have been used in a generic sense and should be amended by the regulator accordingly:

<the Regulator> - the authority responsible for the approval of the development application.

<Regulator's geotechnical DCP> - the appropriate LRM policy title/reference, or Development Control Plan (DCP).

<add reference> - the section or page of the geotechnical report which addresses the item.

<PCA> - the Principal Certifying Authority, or the authority who will be responsible for confirmation of compliance with the development approval conditions.

<tolerable risk> - amend to 'acceptable risk' if that is required by the <Regulator's geotechnical DCP> rather than tolerable.

<Construction Certificate> - the approval necessary to start construction which documents that design has complied with the conditions of approval for the development application.

<Occupation Certificate> - the final approval from the Regulator allowing occupation of the development once all required conditions of consent have been shown to be satisfied.

<Subdivision Certificate> - the final approval from the Regulator confirming that subdivision works have been completed in accordance with the conditions of consent such that development on individual lots may proceed.

<Building Certificate> - a certificate issued by the Regulator confirming that either existing development is in accordance with the Regulator's requirements, or confirming that the Regulator is not aware of any non-compliance which will require rectification works.

ACKNOWLEDGEMENT

These example forms have been based on the forms included in the Wollongong City "Geotechnical Development Control Plan - Development of Sites which may be subject to Slope Instability", effective from 12 July 2006 - with their kind permission. Copies of the Word documents may be obtained from AGS by regulators wishing to prepare their own forms.

FORM	A	Page 1 of 2	
Office Use Only		Geotechnical Declaration and Verification Development Application	
		Regulator: <Add in or change to appropriate name>	
To be submitted with a development application. If this form is not submitted with the geotechnical report the report will be refused. This form is essential to verify that the geotechnical report has been prepared in accordance with <Regulator's geotechnical DCP> and that the author of the geotechnical report is a geotechnical engineer or engineering geologist as defined by <Regulator's geotechnical DCP>. Alternatively, where a geotechnical report has been prepared for subdivision or is greater than two years old or by a professional person not recognised by <Regulator's geotechnical DCP>, then this form may be used as technical verification of the geotechnical report if signed by a geotechnical engineer or engineering geologist as defined by <Regulator's geotechnical DCP>.			
Section 1 Related Application			
Reference		What is the Council development application number?	
DA Site Address			
DA Applicant			
Section 2 Geotechnical Report			
Details		Title:	
		Author's Company/ Organisation Name:	Report Reference No:
		Author:	Dated: / /
Section 3 Checklist			
Geotechnical Requirements (Tick as appropriate, either Yes or No)		The following checklist covers the minimum requirements to be addressed in a geotechnical report. This checklist is to accompany the report. Each item is to be cross-referenced to the section or page of the geotechnical report which addresses that item.	
Yes	No	A review of readily available history of slope instability in the site or related land as per <Add reference>	
☐	☐	An assessment of the risk posed by all reasonably identifiable geotechnical hazards as per <Add reference>	
☐	☐	Plans and sections of the site and related land as per <Add reference>	
☐	☐	Presentation of a geological model as per <Add reference>	
☐	☐	Photographs and/or drawings of the site as per <Add reference>	
☐	☐	A conclusion as to whether the site is suitable for the development proposed to be carried out either conditionally or unconditionally as per <Add reference>	
☐	☐	If any items above are ticked No, an explanation is to be included in the report to justify why. <Add reference>	
Yes No		Subject to recommendations and conditions relevant to:	
☐	☐	selection and construction of footing systems,	
☐	☐	earthworks,	
☐	☐	surface and sub surface drainage,	
☐	☐	recommendations for the selection of structural systems consistent with the geotechnical assessment of the risk,	
☐	☐	any conditions that may be required for the ongoing mitigation and maintenance of the site and the proposal, from a geotechnical viewpoint,	
☐	☐	highlighting and detailing the inspection regime to provide the <PCA> and builder with adequate notification for all necessary inspections.	
☐	☐	State Design life adopted: Years	

Note: <Add reference>: Add in the relevant section or page number of the listed geotechnical report which addresses each item.

FORM	A	Page 2 of 2			
Geotechnical Declaration and Verification Development Application					
Section 4 List of Drawings referenced in Geotechnical Report					
Design Documents	Description	Plan or Document No.	Revision or Version No.	Date	Author
Section 5 Declaration					
Declaration (Tick all that apply) Yes ☐ No ☐ ☐ N/A ☐ ☐ N/A ☐ ☐ No ☐ ☐ No ☐ ☐ No ☐	I am a geotechnical engineer or engineering geologist as defined by the <Regulator's geotechnical DCP> and on behalf of the company below, I: am aware that the geotechnical report I have either prepared or am technically verifying (referenced above) is to be submitted in a support of a development application for the proposed development site (referenced above) and its findings will be relied upon by <the Regulator> in determining the development application. prepared the geotechnical report referenced above in accordance with the AGS (2007c) as amended and <Regulator's geotechnical DCP>. am willing to technically verify that the Geotechnical Report referenced above has been prepared in accordance with the AGS (2007c) as amended and <Regulator's geotechnical DCP>. am willing to technically verify that the geotechnical report prepared for the development application for the site confirms the land will achieve the level of <tolerable risk> of slope instability as a result of the considerations described in <add reference to specific section of> <Regulator's geotechnical DCP> taking into account the total development and site disturbances proposed. am willing to technically verify that the geotechnical report prepared for the site and related land being greater than two years old confirms the land will achieve the level of <tolerable risk> of slope instability as a result of the considerations described <add reference to specific section of> of <Regulator's geotechnical DCP> taking into account the total development and site disturbances proposed. have professional indemnity insurance in accordance with <Regulator's geotechnical DCP> of not less than \$.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <the Regulator>.				
Section 6 Geotechnical Engineer or Engineering Geologist Details					
Company/ Organisation Name					
Name (Company Representative)	Surname:		Mr /Mrs /Other:		
	Given Names:				
	Chartered Professional Status:		Registration No:		
Signature			Dated: / /		

Reference: AGS (2007c) "Practice Note Guidelines for Landslide Risk Management". Australian Geomechanics Society, Australian Geomechanics, V42, N1, March 2007.

Note: N/A = Not Applicable.

FORM	B	Page 1 of 2				
Office Use Only		Structural/Civil/Geotechnical Engineering Declaration – <Construction Certificate> Application				
		Regulator: <Add in or change to appropriate name>				
To be submitted with the structural design forming part of an application for a <construction certificate>. This form must be attached with the submission of the structural documentation required for the determination of a <construction certificate> or combined development application and <construction certificate> submission. This form is essential, as it provides evidence to the <PCA> determining the <construction certificate>, that the structural design has been prepared or verified by a structural engineer or civil engineer as defined by <Regulator's geotechnical DCP> and that the structural design has been prepared in accordance with the recommendations given in the geotechnical report for the same development. This form also covers additional design documents required to cover other works not shown on the main structural/civil design drawings. This form is also essential to establish that the recommendations given in the geotechnical report have been interpreted and incorporated into the structural design as originally intended by the geotechnical engineer in preparing the geotechnical report.						
Section 1 Related Application						
Reference		What is the <Regulator's> development application number?				
DA Site Address						
DA Applicant						
Section 2 Structural/Civil Design Documents						
List of Structural/Civil Design Documents (More space on page two if required)		Description	Plan or Document No.	Revision or Version No.	Date	Author
Section 3 Geotechnical Report						
Details		Title:				
		Author:		Dated: / /		
		Author's Company/ Organisation Name:		Report Reference No:		
Section 4 Declaration by Structural/Civil Engineer or Designer of Additional Design Documents in Relation to a Geotechnical Report						
Declaration (Tick all that apply) Yes No ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐		I am a structural or civil engineer as defined by the <Regulator's geotechnical DCP> and on behalf of the company below.				
		I have prepared the structural designs listed in Section 2 above and/or Section 6 below, in accordance with the recommendations given in the above geotechnical report.				
		I am a design engineer and have prepared Additional Design documents listed in Section 7 below in accordance with the recommendations given in the above geotechnical report.				
		I am aware that the <PCA> will rely on this declaration in granting a <construction certificate> for works to which the above structural design documents and geotechnical report relate.				
		I certify that any residential structure designed or erected in accordance with the structural design prepared by the structural engineer or civil engineer achieves the performance requirements of Clause 1.3 of the current version of AS 2870 (this must be ticked when accompanied by minimal impact certification).				
		I have professional indemnity insurance in accordance with <Regulator's geotechnical DCP> of not less than \$.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <the Regulator>.				

FORM	B	Page 2 of 2				
Structural/Civil/Geotechnical Engineering Declaration – <Construction Certificate> Application						
Section 5 Structural/Civil/Design Engineer Details						
Company/ Organisation Name						
Name (Company Representative)		Surname:		Mr /Mrs /Other:		
		Given:				
		Chartered Professional Status:		Registration No:		
Signature					Dated: / /	
Section 6 Ancillary Structural/Civil Design Required Prior to Completion of Geotechnical Declaration						
List of Structural Design Documents Required	Description	Company Responsible	Plan or Document No.	Revision or Version No.	Date of Additional Form B *	Author
	eg. Landscaping retaining walls					
	eg. Anchor design					
Section 7 Additional Design Documents Required Prior to Completion of Geotechnical Declaration						
List of Design Documents Required	Description	Company	Plan or Document No.	Revision or Version No.	Date of Additional Form B *	Author
	eg. Surface & subsoil drainage design					
	eg. Infiltration or effluent disposal					
Section 8 and 9 are not to be completed until each relevant ancillary and additional Form B has been completed and forwarded to the geotechnical engineer/engineering geologist						
Section 8 Declaration in Relation to Structural/Civil Designs and Additional Design Drawings						
Declaration (Tick all that apply) Yes No ☐ ☐ ☐ ☐ ☐ ☐		I am a geotechnical engineer or engineering geologist as defined by the <Regulator's geotechnical DCP> and on behalf of the company below: I prepared and/or technically verified the above geotechnical report and now declare that I have viewed the above listed design documents prepared for the same development. I am satisfied that the recommendations given in the above geotechnical report have been incorporated into the design documents as intended. I consider no additional drawings are required to show all the required works listed in the Geotechnical Report.				
Section 9 Geotechnical Engineer or Engineering Geologist Details						
Company/ Organisation Name						
Name (Company Representative)		Surname:		Mr /Mrs /Other:		
		Given Names:				
		Chartered Professional Status:		Registration No:		
Signature					Dated: / /	

Note: * A separate Form B is required to be completed by the design engineer for those works listed in each of Sections 6 and 7 of this Form B.

FORM	C	Geotechnical Declaration		Page 1 of 2
		Subdivision <Construction Certificate> Application		
		Office Use Only		Regulator: <Add in or change to appropriate name>
To be submitted with an application for an engineering <construction certificate> for subdivision of land. This form must be attached to the application for the <construction certificate>. This form is essential to verify that the geotechnical report has been prepared in accordance with <Regulator's geotechnical DCP> and that the author of the geotechnical report is a geotechnical engineer or engineering geologist as defined by <Regulator's geotechnical DCP>. Alternatively, where a geotechnical report has been prepared by a professional person not recognised by the <Regulator's geotechnical DCP>, then this form may be used as technical verification of the geotechnical report if signed by a geotechnical engineer or engineering geologist as defined by <Regulator's geotechnical DCP>.				
Section 1 Related Application				
Reference		What is the Regulator's Development Application Number?		
DA Site Address				
DA Applicant				
Section 2 Geotechnical Report				
Details		Title:		
		Author:	Dated: / /	
		Author's Company/ Organisation Name:	Report Reference No:	
Section 3 Declaration				
Declaration (Tick all that apply) Yes No ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐		I am a geotechnical engineer or engineering geologist as defined by the <Regulator's geotechnical DCP> and on behalf of the company below: I prepared the geotechnical report referenced above in accordance with the AGS (2007c) as amended and the <Regulator's geotechnical DCP>. I am willing to technically verify that the geotechnical report referenced above has been prepared in accordance with the AGS (2007c) as amended and <Regulator's geotechnical DCP>. I have professional indemnity insurance in accordance with <Regulator's geotechnical DCP> of not less than \$.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <the Regulator>. I am aware that the geotechnical report I have either prepared or am technically verifying (referenced above) is to be submitted in support of an engineering <construction certificate> for subdivision of land for the proposed development site (referenced above) and its findings will be relied upon by <the Regulator> determining the engineering <construction certificate>.		

FORM	C	Page 2 of 2	
		Geotechnical Declaration Subdivision <Construction Certificate> Application	
Section 4		Checklist	
Geotechnical Requirements (Tick as appropriate, either Yes or No)		The following checklist covers the minimum requirements to be addressed in a geotechnical report in accordance with <Add reference to specific section of> <Regulator's geotechnical DCP>. This checklist is to accompany the report.	
Yes	No		
☐	☐	The extent and stability of proposed embankments including those acting as retarding basins <Add reference>	
☐	☐	Recommended Geotechnical testing requirements <Add reference>	
☐	☐	Required level of geotechnical supervision for each part of the works as defined under AS3798 – Guidelines on Earthworks for Commercial and Residential Developments <Add reference>	
☐	☐	Compaction specification for all fill within private subdivisions <Add reference>	
☐	☐	The level of risk to existing adjacent dwellings as a result of a construction contractor using vibratory rollers anywhere within the site the subject of these works. In the event that vibratory rollers could affect adjacent dwellings, 'high risk' areas shall be identified on a plan and the engineering plans shall be amended to indicate that no vibratory roller shall be used within that zone <Add reference>	
☐	☐	The impact of the installation of services on overall site stability and recommendations on short term drainage methods, shoring requirements and other remedial measures that may be appropriate during installation <Add reference>	
☐	☐	The preferred treatment of any areas of unacceptable risk within privately owned allotments <Add reference>	
☐	☐	Requirement for subsurface drainage lines <Add reference>	
☐	☐	Overall suitability of the engineering plans for the proposed development <Add reference>	
☐	☐	Risk mitigation plan defined <Add reference>	
Section 5		Geotechnical Engineer or Engineering Geologist Details	
Company/ Organisation Name			
Name (Company Representative)	Surname:		Mr /Mrs /Other:
	Given Names:		
	Chartered Professional Status:		Registration No:
Signature			Dated: / /

Reference: AGS (2007c) "Practice Note Guidelines for Landslide Risk Management". Australian Geomechanics Society, Australian Geomechanics, V42, N1, March 2007.

Note: <Add reference>: Add in the relevant section or page number of the listed geotechnical report which addresses each item.

FORM	D	Page 1 of 2				
Geotechnical Declaration Minor Impact						
Office Use Only					Regulator: <Add in or change to appropriate name>	
This form may be used where minor construction works present minimal or no geotechnical impact on the site or related land. A geotechnical engineer or engineering geologist must inspect the site and/or review the proposed development documentation to determine if the proposed development requires a geotechnical report to be prepared to accompany the development application. Where the geotechnical engineer determines that such a report is not required then they must complete this form and attach design recommendations where required. A copy of this form with design recommendation, if required, must be submitted with the development application. Note: In all situations, this form will need to be accompanied by Form B where the structural engineer or civil engineer certifies that any residential structure designed or erected in accordance with the plans and specifications prepared by the structural engineer or civil engineer achieve the performance requirements of Clause 1.3 of the current version of AS 2870. Note: The use of this form does not preclude the geotechnical consultant from requiring a Geotechnical Report.						
Section 1 Related Application						
Reference		What is the Council Development Application Number?				
DA Site Address						
DA Applicant						
Section 2 Documentation						
List of Documents Reviewed (More space on page two if required)		Description	Plan or Document No.	Revision or Version No.	Date	Author
Section 3 Declaration						
Declaration (Tick all that apply)		I am a geotechnical engineer or engineering geologist as defined by the <Regulator's geotechnical DCP> and I have inspected the site and reviewed the proposed development at the DA Site Address described above. As a result of my consideration of the <Regulator's geotechnical DCP>, of my site inspection and review of the documentation listed above, I have determined and declare that, on behalf of the company below: The current load-bearing capacity of the site will not be exceeded or be adversely impacted on by the proposed development, and The proposed works are of such a minor nature that the requirement for geotechnical advice in the form of a geotechnical report, prepared in accordance with <Regulator's geotechnical DCP> is considered unnecessary for the adequate and safe design of the structural elements to be incorporated into the new works as there is no change to the current landslide risk on the site in accordance with AGS (2007c), and In accordance with AS 2870 Residential Slabs and Footings, the site is to be classified as a type: _____ I have attached design recommendations to be incorporated in the structural design in accordance with this site classification. I have professional indemnity insurance in accordance with <Regulator's geotechnical DCP> of not less than \$.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <the Regulator>. I am aware that this declaration shall be used by <The Regulator> as an essential component in granting development consent for a structure to be erected on the site or related land without requiring submission of a geotechnical report complying with the <Regulator's geotechnical DCP> in support of the development application.				
Yes No ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐						

Reference: AGS (2007c) "Practice Note Guidelines for Landslide Risk Management". Australian Geomechanics Society, Australian Geomechanics, V42, N1, March 2007.

FORM	D	Geotechnical Declaration					Page 2 of 2
		Minor Impact					
Section 4 Additional Documentation							
<i>List of Documents Reviewed</i>	Description	Plan or Document No.	Revision or Version No.	Date	Author		
Section 5 Geotechnical Engineer or Engineering Geologist Details							
<i>Company/ Organisation Name</i>							
Name (Company Representative)	Surname:		Mr /Mrs /Other:				
	Given Names:						
	Chartered Professional Status:			Registration No:			
Signature				Dated: / /			

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

FORM	E	Geotechnical Declaration Remediation		Page 1 of 2
		Office Use Only		Regulator: <Add in or change to appropriate name>
This form must be submitted where development must be staged for geotechnical reasons and remediation of the site to a <tolerable risk> is necessary prior to any further development continuing on the site. This form is essential, as it provides verification at each stage of the development, prior to the next stage commencing, that the remediation of the site to a <tolerable risk> has been carried out in accordance with the requirements of the geotechnical report and <add reference to specific section> of <Regulator's geotechnical DCP> and that no unforeseen ground conditions have been encountered which could impact on the integrity of structures on site or related land or the landslide risk. The geotechnical engineer or engineering geologist who prepared and/or verified the report must carry out site inspections as determined by the report to ensure that the design(s) documented on Form(s) B have been completed prior to signing this form.				
Section 1 Related Application				
Reference		What is the Development Application number?		
DA Site Address		Development Stage (s):		
DA Applicant				
Section 2 Geotechnical Report				
Details		Title:		
		Author:		Dated: / /
		Author's Company/ Organisation Name:		Report Reference No:
Section 3 Declaration				
Declaration (Tick all that apply)		I am a geotechnical engineer or engineering geologist as defined by the <Regulator's geotechnical DCP> and, on behalf of the company below:		
Yes No				
☐ ☐		I inspected and am satisfied that the foundation materials upon which the structural elements of the development have been erected, complied with the requirements and recommendations specified in the geotechnical report for Stage (s) <add> of the development.		
☐ ☐		To the best of my knowledge, I am satisfied that Stage(s) <add> of the development referred to above have been carried out in accordance with all the requirements and recommendations of the above geotechnical report, and conditions of development consent relating to geotechnical issues.		
☐ ☐		To the best of my knowledge, I am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the above geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions or site reports issued by me as listed below.		
☐ ☐		I am aware that the <PCA> requires this certificate at the end of stage of the development specified in the development approval and prior to any further development continuing on the site and related land.		
☐ ☐		I am willing to technically verify that the site or related land will now achieve the level of <tolerable risk> of slope instability as defined by <Regulator's geotechnical DCP>.		
☐ ☐		I have professional indemnity insurance in accordance with <Regulator's geotechnical DCP> of not less than \$... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <the Regulator>.		

Note: <add> relevant stage numbers to be inserted.

FORM	E	Geotechnical Declaration Remediation						Page 2 of 2
Section 4 List of Site Instructions and/or Site Reports Issued								
<i>List of Documents Issued</i>	Description/Title	Reference No.	Date	Author	Associated Design Drawings (tick as appropriate)			
					Yes	No		
Section 5		Geotechnical Engineer or Engineering Geologist Details						
<i>Company/ Organisation Name</i>								
Name (Company Representative)	Surname:		Mr /Mrs /Other:					
	Given Names:							
	Chartered Professional Status:		Registration No:					
Signature		Dated: / /						

FORM	F	Page 1 of 2				
Office Use Only		Geotechnical Declaration Final Structural/Civil Certificate		Regulator: <Add in or change to appropriate name>		
This form must be submitted to the <PCA> at the completion of a project and prior to the issue of an <occupation certificate>. This form is essential, as it provides evidence to the <PCA> that the development works have been carried out in accordance with the requirements of the structural design, any site inspections, and that any changes to the development occurring during construction, were carried out in accordance with all the requirements and recommendations of the structural design and geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions issued.						
Section 1 Related Application						
Reference		What is <the Regulator's> Development Application number?				
DA Site Address						
DA Applicant						
Section 2 Geotechnical Report						
Details		Title:				
		Author:		Dated: / /		
		Author's Company/ Organisation Name:		Report Reference No:		
Section 3 Structural Civil Design Documents appropriate to the 'as constructed' development						
List of Structural Civil Design Documents (More space on page two if required)		Description	Plan or Document No.	Revision or Version No.	Date	Author
Section 4 Declaration						
Declaration (Tick all that apply) Yes No ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐		I am a structural or civil engineer as defined by the <Regulator's geotechnical DCP> and I prepared the above structural designs in accordance with the recommendations given in the geotechnical report described above on behalf of the company below. I:				
		inspected and am satisfied that the structural elements of the above development have been erected, and complied with the requirements and recommendations specified in the structural design and geotechnical report.				
		to the best of my knowledge, am satisfied that the above development has been carried out in accordance with all the requirements and recommendations of the structural design and above geotechnical report, and conditions of development consent relating to geotechnical issues.				
		to the best of my knowledge, am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the structural design and above geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions issued by me as listed below.				
		am aware that the <PCA> requires this certificate prior to issuing an <occupation certificate> for the above development and will rely on this certificate as verification that the above development has been erected, and complied with the requirements and recommendations specified in the structural design and geotechnical report as defined by <Regulator's geotechnical DCP> and in determining the <occupation certificate>.				
		have professional indemnity insurance in accordance with <Regulator's geotechnical DCP> of not less than \$... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <the Regulator>.				

FORM F	Geotechnical Declaration Final Structural/Civil Certificate						Page 2 of 2
	Section 5 List of Site Instructions Issued						
<i>List of Documents Issued</i>	Description/Title	Reference No.	Date	Author	Associated Design Drawings		
					Yes	No	
Section 6 Additional Design Documents							
<i>List of Additional Design Documents</i>	Description	Plan or Document No.	Revision or Version No.	Date	Author		
Section 7 Structural Engineer or Civil Engineer Details							
<i>Company/ Organisation Name</i>							
<i>Name (Company Representative)</i>	Surname:		Mr /Mrs /Other:				
	Given Names:						
	Chartered Professional Status:			Registration No:			
<i>Signature</i>				Dated: / /			

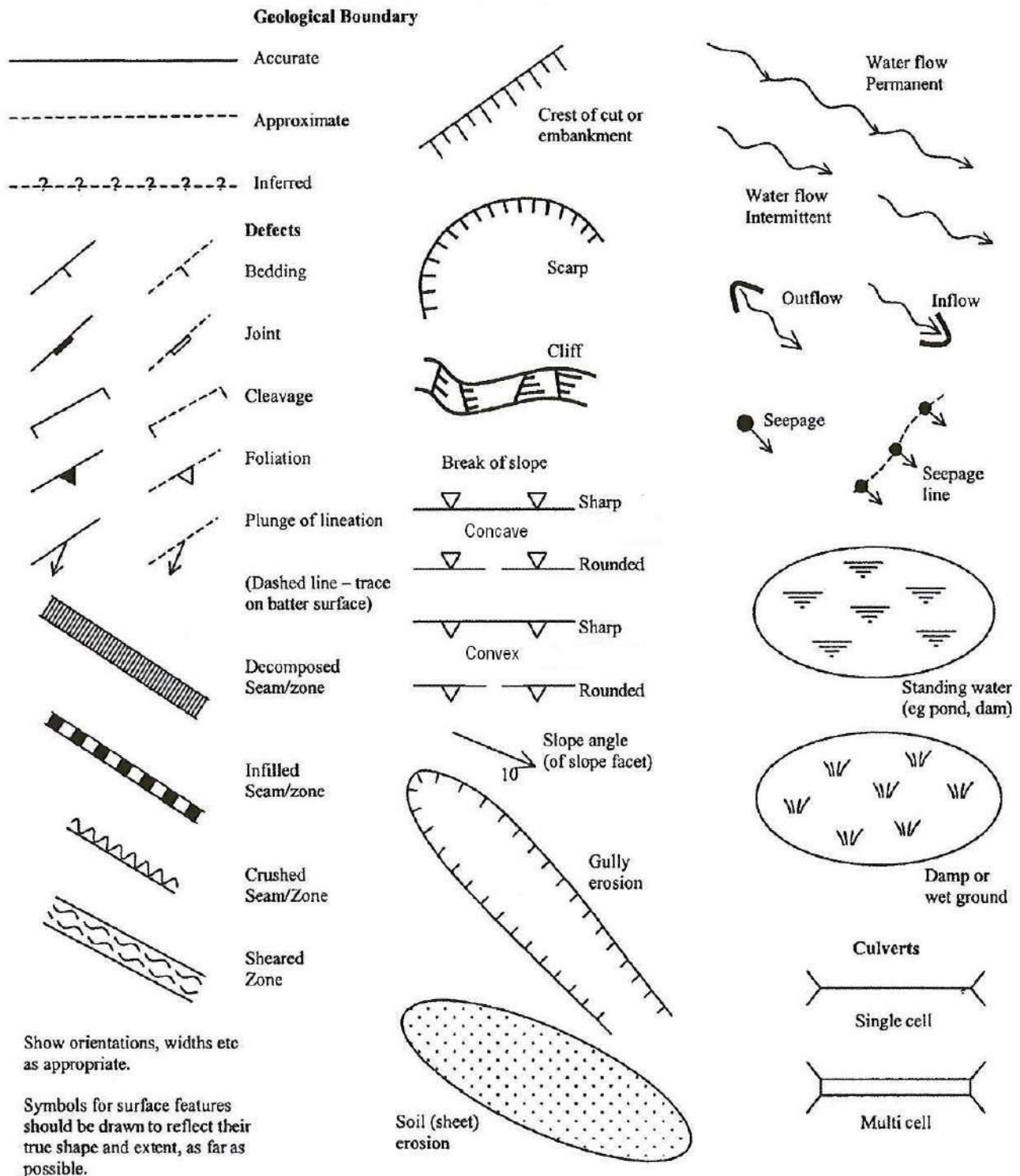
FORM	G	Page 1 of 2				
Office Use Only		Geotechnical Declaration Final Geotechnical Certificate				
		Regulator: <Add in or change to appropriate name>				
This form must be submitted to the <PCA> at the completion of a project and prior to the issue of an <occupation or subdivision certificate>. This form is essential, as it provides verification that the development works have been carried out in accordance with the requirements of the geotechnical report during construction, and any site inspections, and that no unforeseen ground conditions have been encountered which could have an impact on the integrity of structures on site or related land and any subsequent geotechnical requirements introduced during the construction process.						
Section 1 Related Application						
Reference		What is the Development Application number?				
DA Site Address						
DA Applicant						
Section 2 Geotechnical Report						
Details		Title:				
		Author:		Dated: / /		
		Author's Company/ Organisation Name:		Report Reference No:		
Section 3 Work as Executed Drawings & Ongoing Maintenance Plans relevant to Geotechnical Risk Management						
List of Documents (more space on page 2 if required)		Description	Plan or Document No.	Revision or Version No.	Date	Author
Section 4 Declaration						
Declaration (Tick all that apply) Yes No ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐		I am a geotechnical engineer or engineering geologist as defined by the <Regulator's geotechnical DCP> and I prepared or verified the geotechnical report as described above on behalf of the company below. I: inspected and am satisfied that the foundation materials upon which the structural elements of the development have been erected, complied with the requirements and recommendations specified in the geotechnical report. to the best of my knowledge, am satisfied that the development referred to above has been carried out in accordance with all the requirements and recommendations of the above geotechnical report, and conditions of development consent relating to geotechnical issues. to the best of my knowledge, am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the above geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions or site reports issued by me as listed below. am aware that the <PCA> requires this certificate prior to issuing an occupation or subdivision certificate for the above development and will rely on this certificate as verification that the above development has achieved the necessary level of <tolerable risk> as defined by <Regulator's geotechnical DCP> and in determining the <occupation or subdivision certificate>.				

FORM	G	Geotechnical Declaration Final Geotechnical Certificate					
Section 5 List of Site Reports or Site Instructions Issued							
<i>List of Documents Issued</i>	Description/Title	Reference No.	Date	Author	Associated Design Drawings		
					Yes	No	
Section 6 Additional Work as Executed Drawings and Ongoing Maintenance Plans relevant to Geotechnical Risk Management							
<i>List of Additional Documents</i>	Description	Plan or Document No.	Revision or Version No.	Date	Author		
Section 7 Geotechnical Engineer or Engineering Geologist Details							
<i>Company/ Organisation Name</i>							
<i>Name (Company Representative)</i>	Surname:		Mr /Mrs /Other:				
	Given Names:						
	Chartered Professional Status:			Registration No:			
<i>Signature</i>				Dated: / /			

FORM	H	Geotechnical Declaration <Building Certificate> or Order		Page 1 of 2
		Office Use Only		Regulator: <Add in or change to appropriate name>
This form is to be submitted with Application for a <Building Certificate> or in response to an order.				
Section 1 Related Application				
Reference		What is the Regulator's DA / BA / Order number?		
Site Address				
Applicant				
Section 2 Geotechnical Report				
Details		Title:		
		Author:	Dated: / /	
		Author's Company/ Organisation Name:	Report Reference No:	
Section 3 Declaration				
Declaration (Tick all that apply) Yes No ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐ ☐		I am a geotechnical engineer or engineering geologist as defined by the <Regulator's geotechnical DCP> and I prepared or verified the geotechnical report as described above on behalf of the company below. I: have inspected the site and existing development and am satisfied that both the site and development achieves <tolerable risk> level requirement of the <Regulator's geotechnical DCP>. The attached report provides details of the assessment in accordance with the <Regulator's geotechnical DCP>. The report also contains recommendations as to any reasonable and practical measures that can be undertaken to reduce foreseeable risk. have inspected the site of the existing development. The attached report details the remedial actions required to be undertaken prior to me being prepared to certify that the site and the development achieves the <tolerable risk> criteria required by the <Regulator's geotechnical DCP>. to the best of my knowledge, am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the above geotechnical report, conditions of development consent relating to geotechnical issues, and any site reports or site instructions issued by me as listed below. am aware that the <PCA> requires this certificate prior to issuing a <Building Certificate> for the above development and will rely on this certificate as verification that the development has achieved the necessary level of <tolerable risk> as defined by <Regulator's geotechnical DCP> and in determining the <occupation or subdivision certificate>. have professional indemnity insurance in accordance with <Regulator's geotechnical DCP> of not less than \$.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <the Regulator>.		

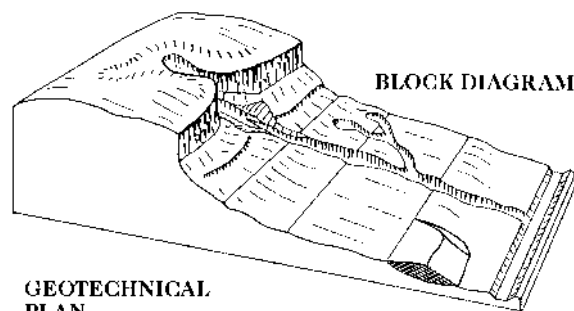
FORM	H	Geotechnical Declaration					Page 2 of 2
		<Building Certificate> or Order					
Section 4 List of Site Reports or Site Instructions Issued							
<i>List of Documents Issued</i>	Description/Title	Reference No.	Date	Author	Associated Design Drawings		
					Yes	No	
Section 5 Geotechnical Engineer or Engineering Geologist Details							
<i>Company/ Organisation Name</i>							
Name (Company Representative)	Surname:			Mr /Mrs /Other:			
	Given Names:						
	Chartered Professional Status:			Registration No:			
Signature				Dated: / /			

APPENDIX E - GEOLOGICAL AND GEOMORPHOLOGICAL MAPPING SYMBOLS AND TERMINOLOGY

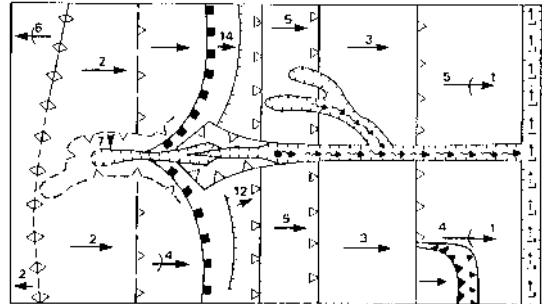


Examples of Mapping Symbols (after Guide to Slope Risk Analysis Version 3.1 November 2001, Roads and Traffic Authority of New South Wales).

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007



**GEOTECHNICAL
PLAN**



SYMBOL	GROUND PROFILE	
	Convex	Well defined or angular break of slope
	Concave	
	Convex	Poorly defined or smooth change of slope
	Concave	
	Breaks of slope	Convex and concave too close together to allow the use of separate symbols
	Changes of slope	
	Sharp	Ridge crest
	Rounded	
	Cliff or escarpment or sharp break 40° or more (estimated height in metres)	
	Uniform slope	Slope direction and angle (Degrees)
	Concave slope	
	Convex slope	
	Top	Cut or fill slope, arrows pointing down slope
	Bottom	
	Hummocky or irregular ground	
	Open drain, unlined	
	Open drain, lined	
	Fenceline	
	Property boundary	
	Dry stone wall	
	Major joint in rock face (opening in millimetres)	
	Tension crack (opening in millimetres)	

Example of Mapping Symbols

(after V Gardiner & R V Dackombe (1983). Geomorphological Field Manual. George Allen & Unwin).

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX F- EXAMPLE OF VULNERABILITY VALUES

SUMMARY OF HONG KONG VULNERABILITY RANGES FOR PERSONS, AND RECOMMENDED VALUES FOR LOSS OF LIFE FOR LANDSLIDING IN SIMILAR SITUATIONS

The following table is adapted from P J Finlay, G R Mostyn & R Fell (1999). *Landslides: Prediction of Travel Distance and Guidelines for Vulnerability of Persons*. Proc 8th. Australia New Zealand Conference on Geomechanics, Hobart. Australian Geomechanics Society, ISBN 1 86445 0029, Vol 1, pp.105-113.

Case	Range in Data	Recommended Value	Comments
Person in Open Space			
If struck by a rockfall	0.1 – 0.7	0.5	May be injured but unlikely to cause death
If buried by debris	0.8 – 1.0	1.0	Death by asphyxia almost certain
If not buried	0.1 – 0.5	0.1	High chance of survival
Persons in a Vehicle			
If the vehicle is buried/crushed	0.9 – 1.0	1.0	Death is almost certain
If the vehicle is damaged only	0 – 0.3	0.3	High chance of survival
Person in a Building			
If the building collapses	0.9 – 1.0	1.0	Death is almost certain
If the building is inundated with debris and the person buried	0.8 – 1.0	1.0	Death is highly likely
If the debris strikes the building only	0 – 0.1	0.05	Very high chance of survival

EXAMPLE OF VULNERABILITY VALUES FOR DESTRUCTION OF PEOPLE, BUILDINGS AND ROADS

The following table is adapted from Marion Michael-Leiba, Fred Baynes, Greg Scott & Ken Granger (2002). *Quantitative Landslide Risk Assessment of Cairns*. Australian Geomechanics, June 2002.

Geomorphic Unit	Vulnerability Values		
	People	Buildings	Roads
Hill slopes	0.05	0.25	0.3
Proximal debris fan	0.5	1.0	1.0
Distal debris fan	0.05	0.1	0.3

EXAMPLE OF VULNERABILITY VALUES FOR LIFE FOR ROCKFALLS AND DEBRIS FLOWS FOR LAWRENCE HARGRAVE DRIVE PROJECT, COALCLIFF TO CLIFTON AREA, AUSTRALIA

The following table is adapted from R A Wilson, A T Moon, M Hendricks & I E Stewart (2005). *Application of quantitative risk assessment to the Lawrence Hargrave Drive Project, New South Wales, Australia*. Landslide Risk Management - Hungr, Fell, Couture & Eberhardt (eds) 2005. Taylor & Francis Group, London, ISBN 04 1538 043X.

Order of magnitude of landslide crossing road (m ³)	Rockfalls from Scarborough Cliff		Debris flow from Northern Amphitheatre	
	Landslide hits car	Car hits landslide	Landslide hits car	Car hits landslide
0.03	0.05	0.006	–	–
0.3	0.1	0.002	–	–
3	0.3	0.03	0.001	–
30	0.7	0.03	0.01	0.001
300	1	0.03	0.1	0.003
3,000	1	0.03	1	0.003

NOTE: The above data should be applied with common sense, taking into account the circumstances of the landslide being studied. Judgment may indicate values other than the recommended value are appropriate for a particular case.

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX G - SOME GUIDELINES FOR HILLSIDE CONSTRUCTION

GOOD ENGINEERING PRACTICE

POOR ENGINEERING PRACTICE

ADVICE

GEOTECHNICAL ASSESSMENT	Obtain advice from a qualified, experienced geotechnical practitioner at early stage of planning and before site works.	Prepare detailed plan and start site works before geotechnical advice.
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PLANNING

SITE PLANNING	Having obtained geotechnical advice, plan the development with the risk arising from the identified hazards and consequences in mind.	Plan development without regard for the Risk.
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DESIGN AND CONSTRUCTION

HOUSE DESIGN	Use flexible structures which incorporate properly designed brickwork, timber or steel frames, timber or panel cladding. Consider use of split levels. Use decks for recreational areas where appropriate.	Floor plans which require extensive cutting and filling. Movement intolerant structures.
SITE CLEARING	Retain natural vegetation wherever practicable.	Indiscriminately clear the site.
ACCESS & DRIVEWAYS	Satisfy requirements below for cuts, fills, retaining walls and drainage. Council specifications for grades may need to be modified. Driveways and parking areas may need to be fully supported on piers.	Excavate and fill for site access before geotechnical advice.
EARTHWORKS	Retain natural contours wherever possible.	Indiscriminatory bulk earthworks.
CUTS	Minimise depth. Support with engineered retaining walls or batter to appropriate slope. Provide drainage measures and erosion control.	Large scale cuts and benching. Unsupported cuts. Ignore drainage requirements
FILLS	Minimise height. Strip vegetation and topsoil and key into natural slopes prior to filling. Use clean fill materials and compact to engineering standards. Batter to appropriate slope or support with engineered retaining wall. Provide surface drainage and appropriate subsurface drainage.	Loose or poorly compacted fill, which if it fails, may flow a considerable distance including onto property below. Block natural drainage lines. Fill over existing vegetation and topsoil. Include stumps, trees, vegetation, topsoil, boulders, building rubble etc in fill.
ROCK OUTCROPS & BOULDERS	Remove or stabilise boulders which may have unacceptable risk. Support rock faces where necessary.	Disturb or undercut detached blocks or boulders.
RETAINING WALLS	Engineer design to resist applied soil and water forces. Found on rock where practicable. Provide subsurface drainage within wall backfill and surface drainage on slope above. Construct wall as soon as possible after cut/fill operation.	Construct a structurally inadequate wall such as sandstone flagging, brick or unreinforced blockwork. Lack of subsurface drains and weepholes.
FOOTINGS	Found within rock where practicable. Use rows of piers or strip footings oriented up and down slope. Design for lateral creep pressures if necessary. Backfill footing excavations to exclude ingress of surface water.	Found on topsoil, loose fill, detached boulders or undercut cliffs.
SWIMMING POOLS	Engineer designed. Support on piers to rock where practicable. Provide with under-drainage and gravity drain outlet where practicable. Design for high soil pressures which may develop on uphill side whilst there may be little or no lateral support on downhill side.	
DRAINAGE		
SURFACE	Provide at tops of cut and fill slopes. Discharge to street drainage or natural water courses. Provide general falls to prevent blockage by siltation and incorporate silt traps. Line to minimise infiltration and make flexible where possible. Special structures to dissipate energy at changes of slope and/or direction.	Discharge at top of fills and cuts. Allow water to pond on bench areas.
SUBSURFACE	Provide filter around subsurface drain. Provide drain behind retaining walls. Use flexible pipelines with access for maintenance. Prevent inflow of surface water.	Discharge roof runoff into absorption trenches.
SEPTIC & SULLAGE	Usually requires pump-out or mains sewer systems; absorption trenches may be possible in some areas if risk is acceptable. Storage tanks should be water-tight and adequately founded.	Discharge sullage directly onto and into slopes. Use absorption trenches without consideration of landslide risk.
EROSION CONTROL & LANDSCAPING	Control erosion as this may lead to instability. Revegetate cleared area.	Failure to observe earthworks and drainage recommendations when landscaping.

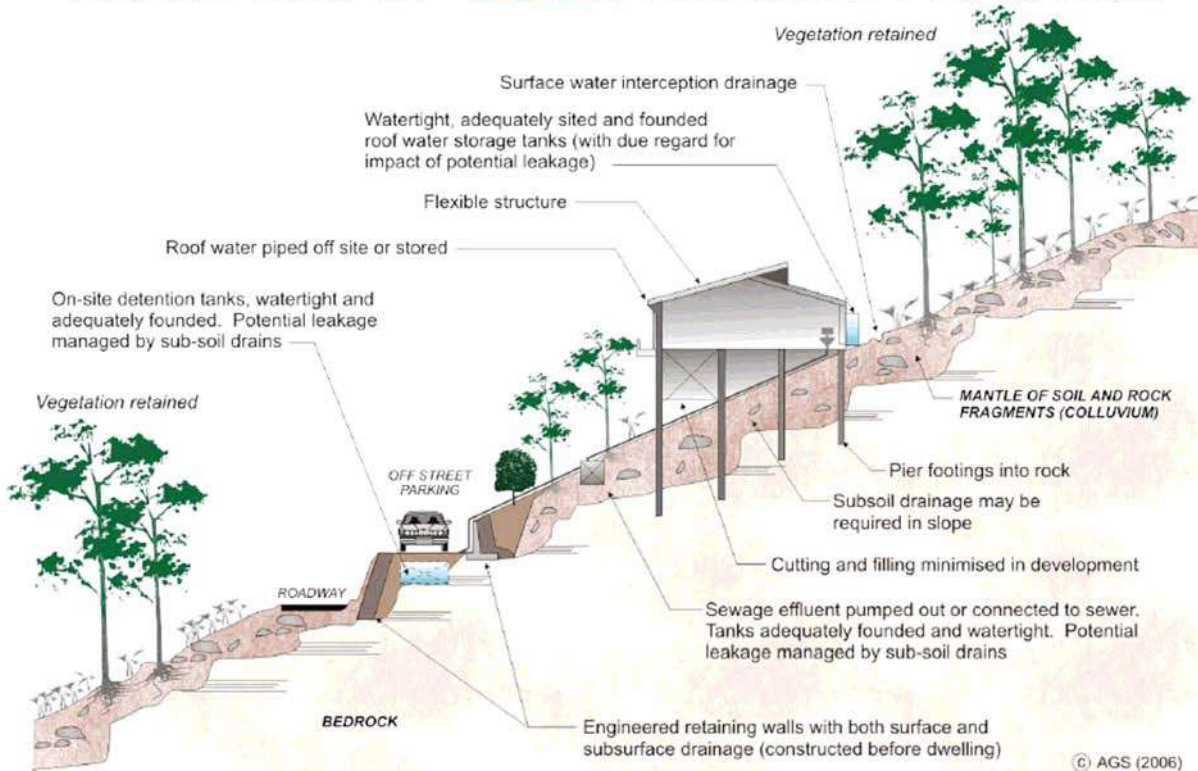
DRAWINGS AND SITE VISITS DURING CONSTRUCTION

DRAWINGS	Building Application drawings should be viewed by geotechnical consultant	
SITE VISITS	Site Visits by consultant may be appropriate during construction/	

INSPECTION AND MAINTENANCE BY OWNER

OWNER'S RESPONSIBILITY	Clean drainage systems; repair broken joints in drains and leaks in supply pipes. Where structural distress is evident see advice. If seepage observed, determine causes or seek advice on consequences.	
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EXAMPLES OF **GOOD** HILLSIDE PRACTICE



EXAMPLES OF **POOR** HILLSIDE PRACTICE

